PT Slab-on-Ground Technical Session 5

Optimizing Shallow Slab-on-Grade Foundation Designs: Efficient

Stiffness vs. Excessive Mass

Speaker: Anna Olveda

and Florian Aalami

8:30am - 9:20am







Evaluating Performance of Ribbed vs Void Formed Shallow Foundations

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Anna Olveda, MSCE VP of Engineering









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Learning Objectives

At the end of this presentation, you will be able to...

- Assess the geometry of the foundation for efficiency
- Evaluate the effects of different tendon layouts
- Understand the effects of stiffness relative to the concrete neutral axis and tendon placement
- Holistic view of construction practices relative to the overall value of the design



Fundamental Drivers of SOG Performance

Independent from applied code, calculation method or soil conditions, the performance of two alternative designs can be evaluated using these factors:

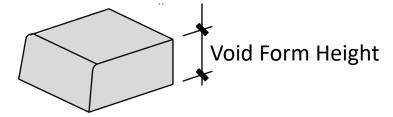
- Bending Stiffness is the #1 determinant of performance
- Precompression provided by PT delays cracking and ensures inherent stiffness can be fully utilized
- Profiled PT can offset applied moments



Parametric Study of Different Geometries

Keeping the moment of inertia stiffness constant, we compare the performance of three different shallow foundation configurations:

- Traditional ribbed slab
- Non collapsable void form foundation
 - o 8.5 in
 - 0 12 in



Sample Project

Common 40 ft x 70 ft footprint

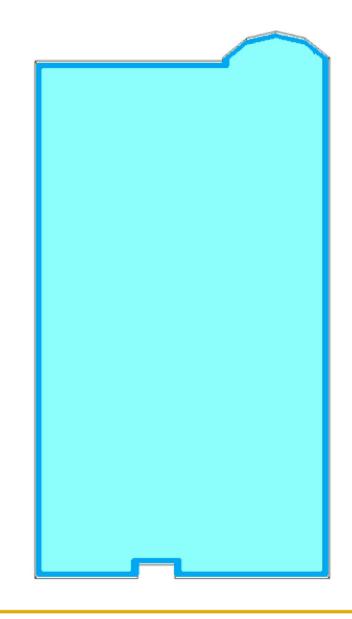
	Center Lift		Edge Lift	
PVR (inches)	E _m (feet)	Y _m (in)	E _m (feet)	Y _m (in)
1	8.3	-0.96	4.2	1.33
4	6.1	-2.33	3.5	3.8

Loading:



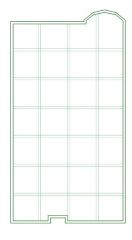
1,200 plf around perimeter

40 psf interior live load



Calibrated Models to Have Same Stiffness

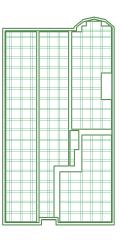
Maintaining minimum stiffness in long direction was governing factor



Traditional – Plan View

$$I_{short} = 161,491 \text{ in}^4$$

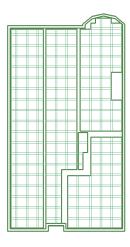
 $I_{short} = 245,897 \text{ in}^4$



8.5" Void Form – Plan View

$$I_{short} = 219,103 \text{ in}^4$$

 $I_{short} = 250,366 \text{ in}^4$



12" Void Form – Plan View

$$I_{short} = 204,444 \text{ in}^4$$

$$I_{short} = 250,034 \text{ in}^4$$

Traditional Ribbed Foundation

<u>Data</u>

Exterior Beam: 10" x 28"

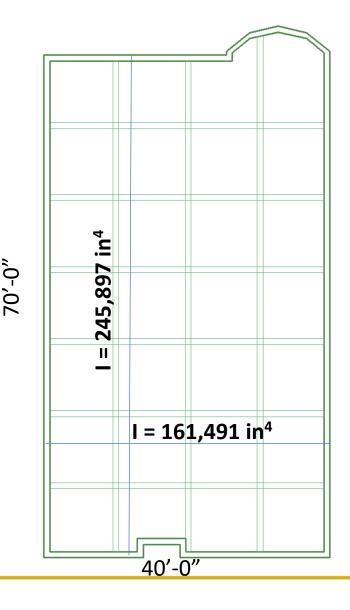
Interior Beam: 10" x 24"

Slab Thickness: 4"

Area: 2,799 ft²

Concrete Volume: 71 cu yds

Concrete Volume with waste factor: 88 cu yds





Traditional Ribbed Foundation

Expectation





8.5 in Voided Slab

Data

Exterior Beam: 10" x 36"

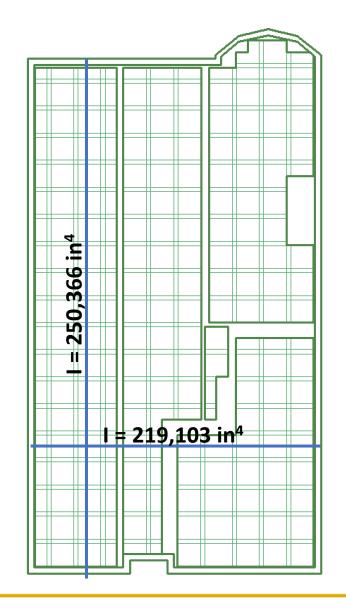
Void Form Boxes = $738 - 8 \frac{1}{2}$ " Boxes

Slab Thickness: 4"

Area: 2,799 ft²

Concrete Volume: 77 cu yds

Concrete Volume with waste factor: 79 cu yds





8.5 in Voided Slab

Expectation

Reality





12 in Voided Slab

Data

Exterior Beam: 10" x 34"

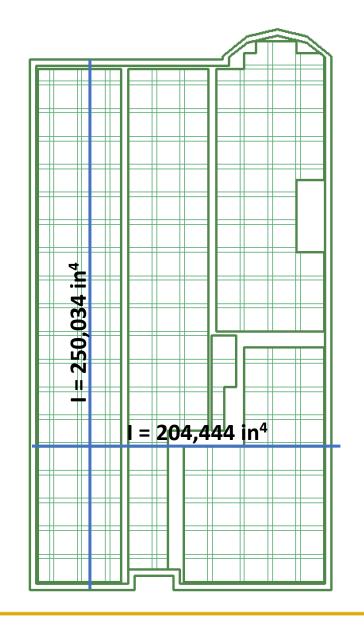
Void Form Boxes = 738 - 12" Boxes

Slab Thickness: 4"

Area: 2,799 ft²

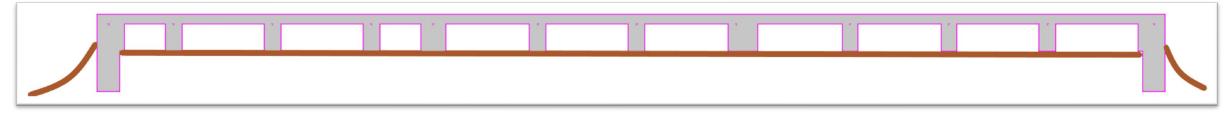
Concrete Volume: 85.5 cu yds

Concrete Volume with waste factor: 87.5 cu yds



12 in Voided Slab

Expectation



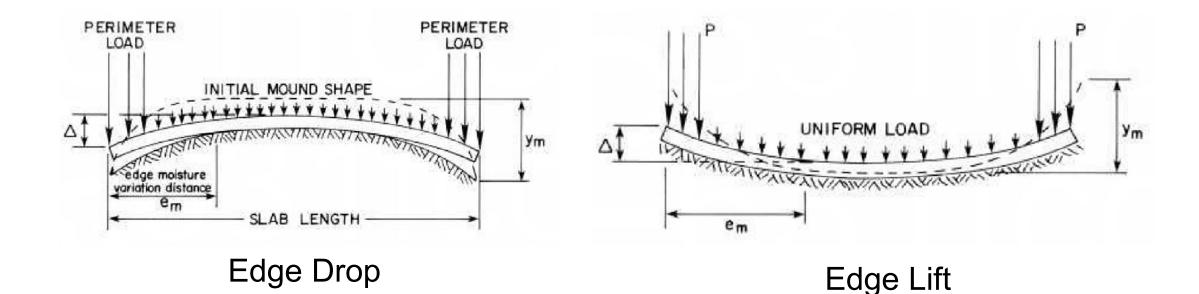
Reality





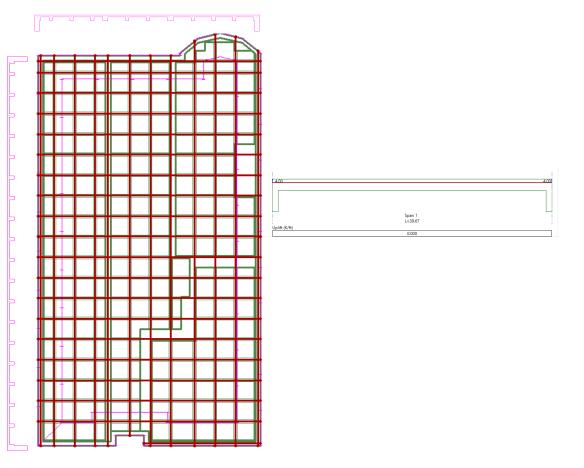
Evaluation of Deflection Performance

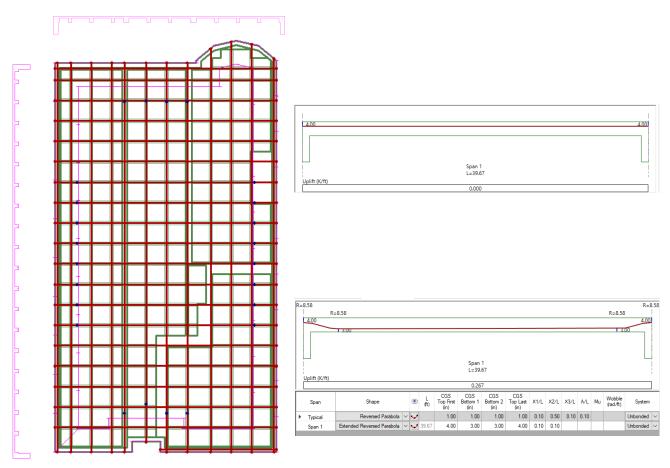
Using Finite Element Analysis, subject all 3 slabs to exact same soil loading conditions:





Tendon Location & Placement



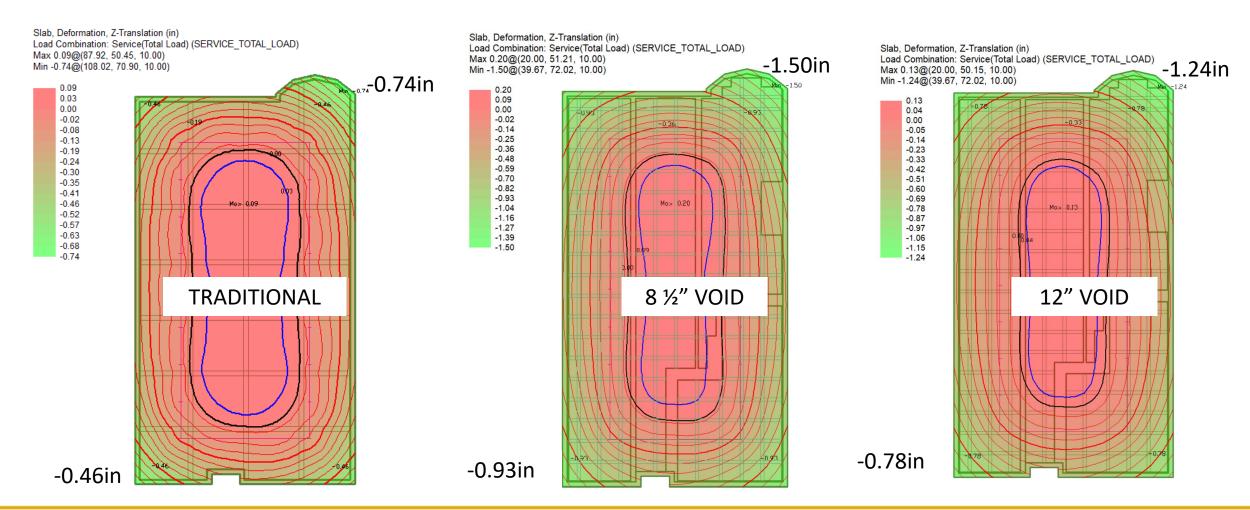


Tendon Layout 1

Tendon Layout 2

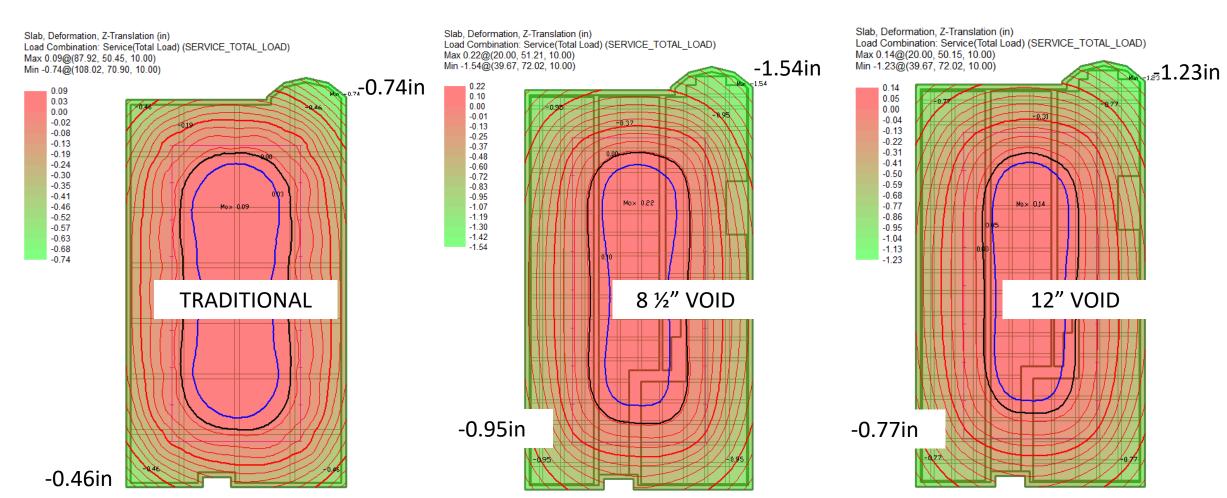


Edge Drop – 1 in – Theoretical Analysis



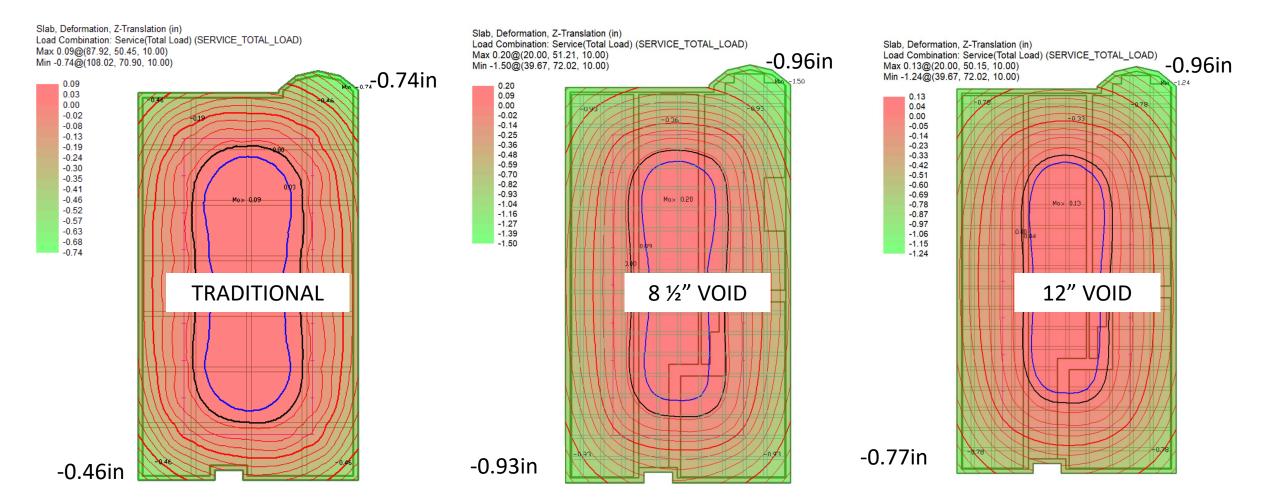


Edge Drop – 1 in – Theoretical Analysis





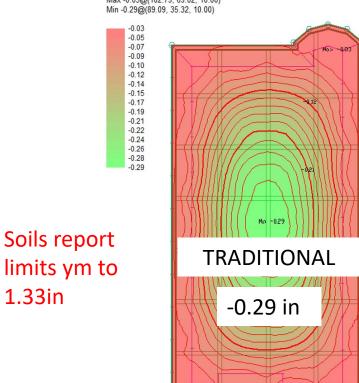
Edge Drop – 1 in – ym Bounded to 0.96in



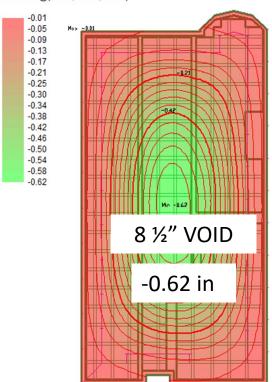


Edge Lift – 1 in

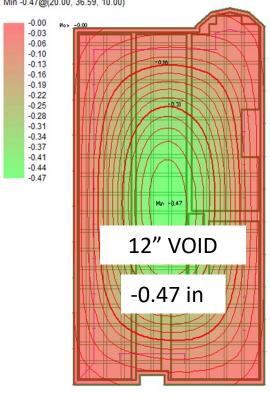
Slab, Deformation, Z-Translation (in) Load Combination: Service(Total Load) (SERVICE_TOTAL_LOAD) Max -0.03@(102.79, 69.02, 10.00)



Slab, Deformation, Z-Translation (in) Load Combination: Service(Total Load) (SERVICE_TOTAL_LOAD) Max -0.01@(0.00, 71.52, 10.00) Min -0.62@(20.00, 36.59, 10.00)



Slab, Deformation, Z-Translation (in) Load Combination: Service(Total Load) (SERVICE_TOTAL_LOAD) Max -0.00@(0.00, 71.52, 10.00) Min -0.47@(20.00, 36.59, 10.00)



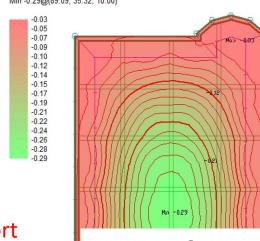


1.33in

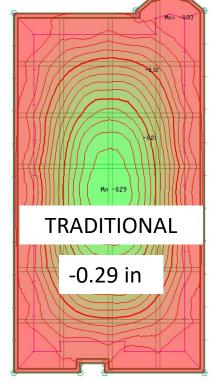


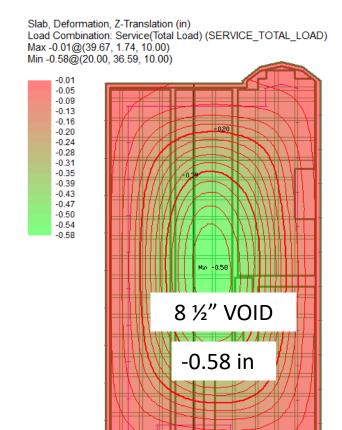
Edge Lift – 1 in

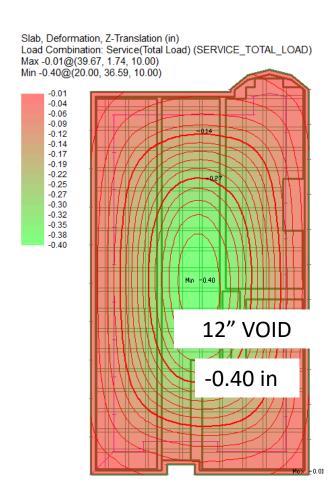
Slab, Deformation, Z-Translation (in) Load Combination: Service(Total Load) (SERVICE_TOTAL_LOAD) Max -0.03@(102.79, 69.02, 10.00) Min -0.29@(89.09, 35.32, 10.00)



Soils report limits ym to 1.33in



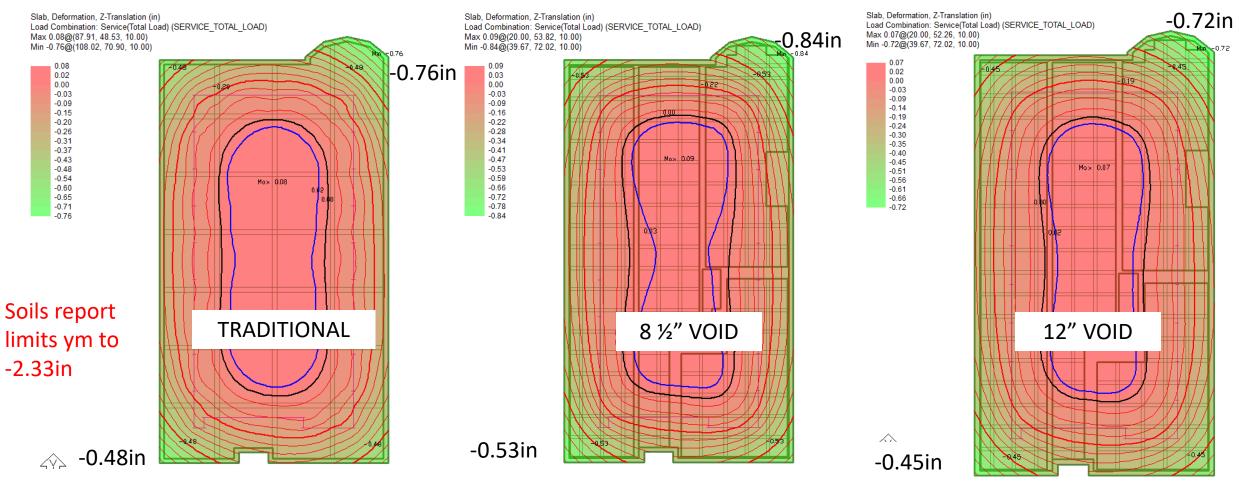






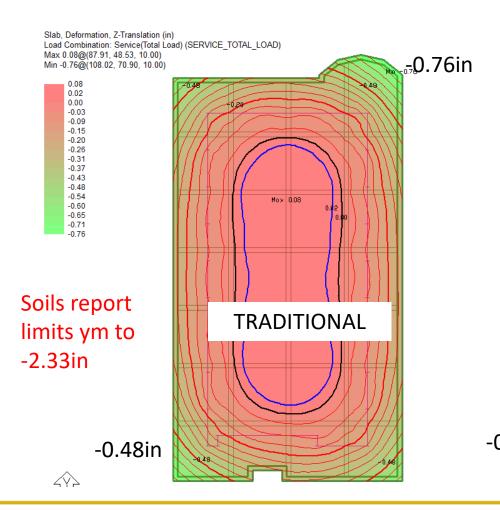


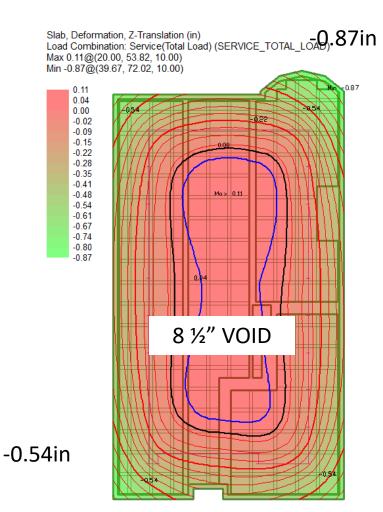
Edge Drop – 4 in

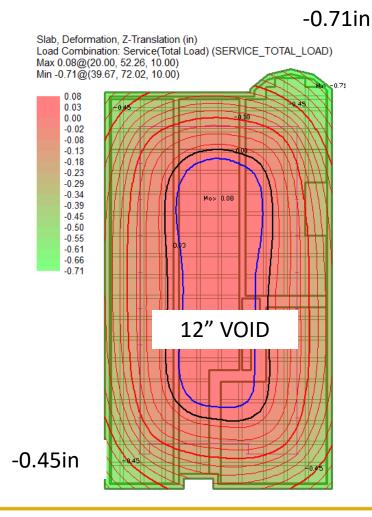




Edge Drop – 4 in



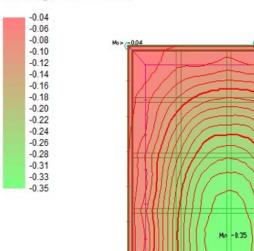




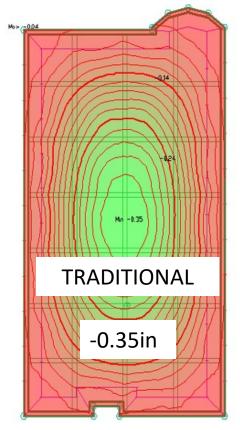


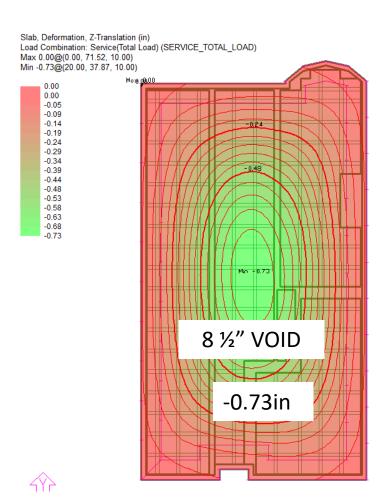
Edge Lift – 4 in

Slab, Deformation, Z-Translation (in) Load Combination: Service(Total Load) (SERVICE_TOTAL_LOAD) Max -0.04@(68.02, 70.40, 10.00) Min -0.35@(89.09, 35.32, 10.00)

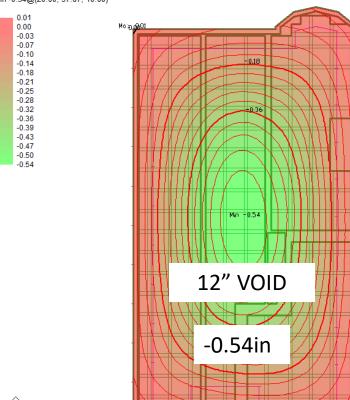


Soils report limits ym to 3.80 in





Slab, Deformation, Z-Translation (in) Load Combination: Service(Total Load) (SERVICE_TOTAL_LOAD) Max 0.01@(0.00, 71.52, 10.00) Min -0.54@(20.00, 37.87, 10.00)

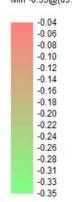




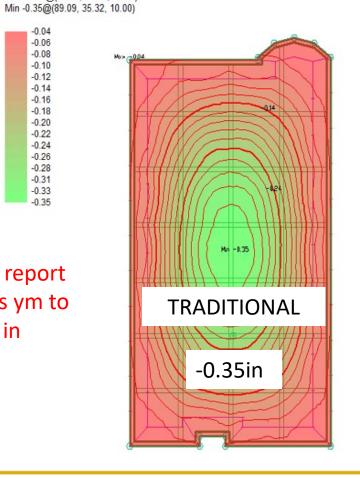


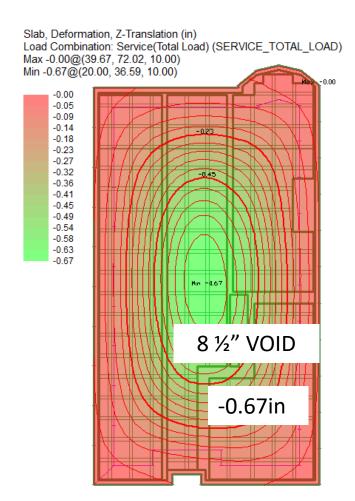
Edge Lift – 4 in

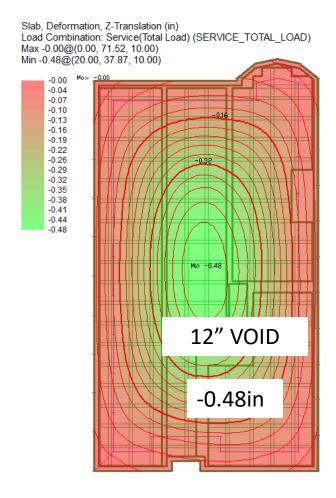
Slab, Deformation, Z-Translation (in) Load Combination: Service(Total Load) (SERVICE_TOTAL_LOAD) Max -0.04@(68.02, 70.40, 10.00)



Soils report limits ym to 3.80 in

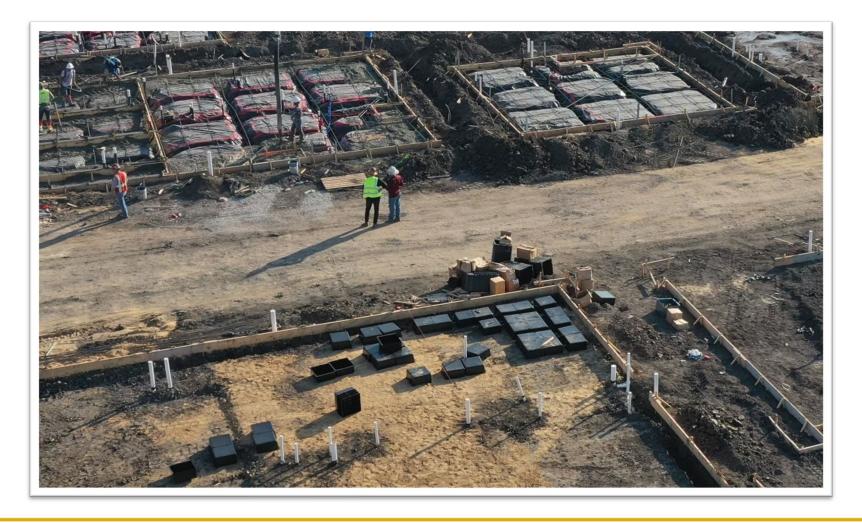








Traditional vs Void Form



Summary of Deflection Comparison

Void Forms Tendon Layout 1

1" PVR (in of max deflection)

	Traditional	8.5in Void	12in Void
Edge Drop	-0.74	-0.96*	-0.96*
Edge Lift	-0.29	-0.62	-0.47

4" PVR (in of max deflection)

	Traditional	8.5in Void	12in Void
Edge Drop	-0.76	-0.84	-0.72
Edge Lift	-0.35	-0.73	-0.55



Summary of Deflection Comparison

Void Forms Tendon Layout 2

1" PVR (in of max deflection)

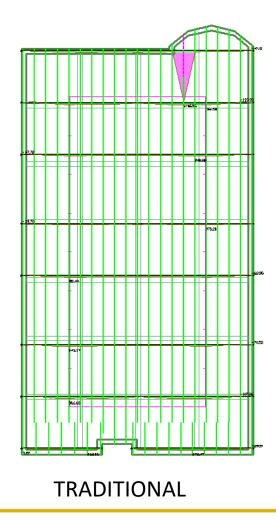
	Traditional	8.5in Void	12in Void
Edge Drop	-0.74	-0.96*	-0.96*
Edge Lift	-0.29	-0.58	-0.40

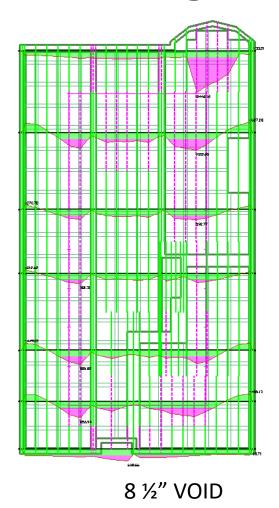
4" PVR (in of max deflection)

	Traditional	8.5in Void	12in Void
Edge Drop	-0.76	-0.84	-0.72
Edge Lift	-0.35	-0.67	-0.48



Top Fiber Stresses – Edge Drop – 1 in

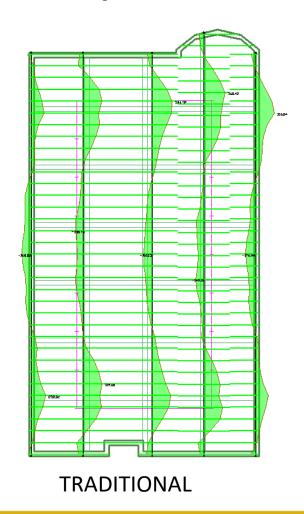








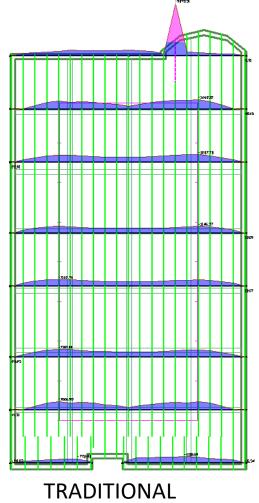
Top Fiber Stresses – Edge Drop – 1 in

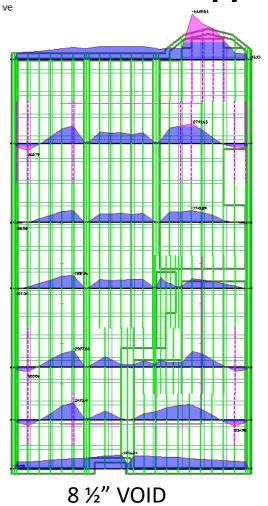


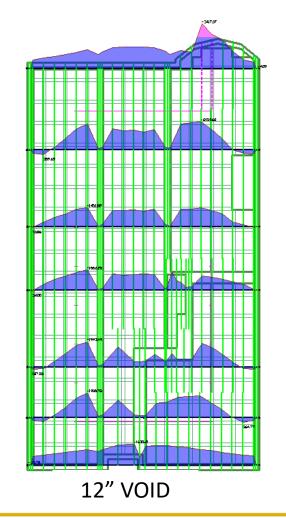




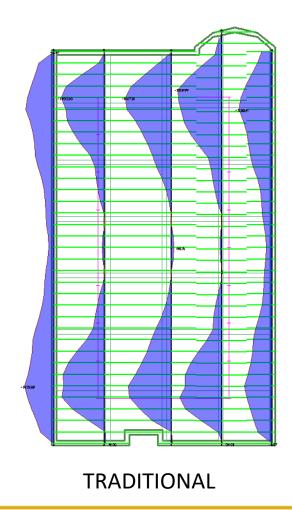
Bottom Fiber Stresses – Edge Drop – 1 in

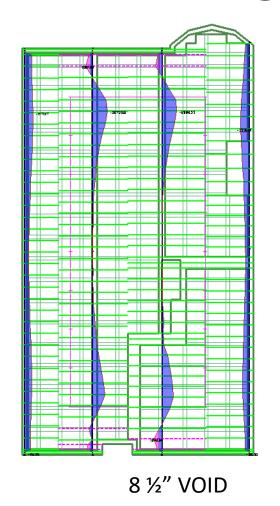


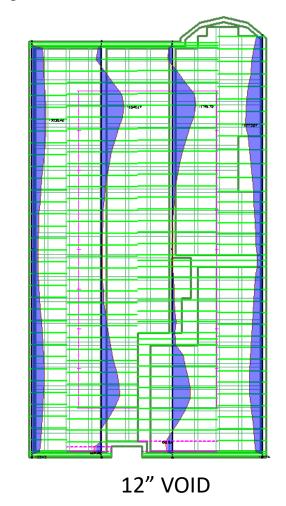




Bottom Fiber Stresses – Edge Drop – 1 in

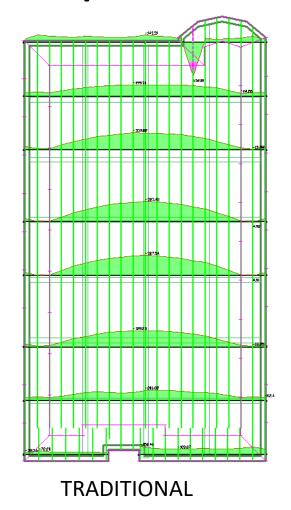


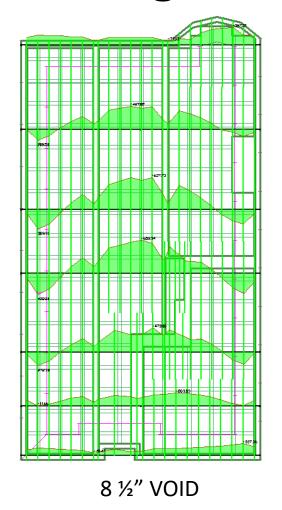


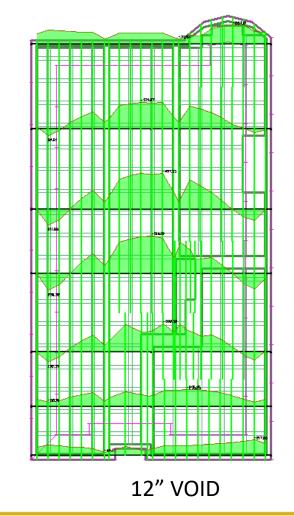




Top Fiber Stresses – Edge Lift – 1 in

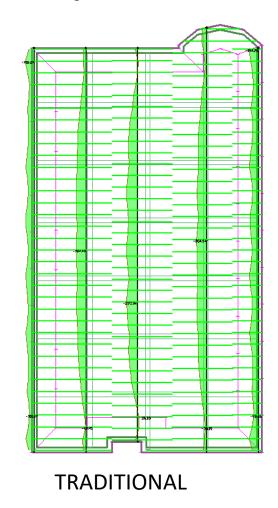




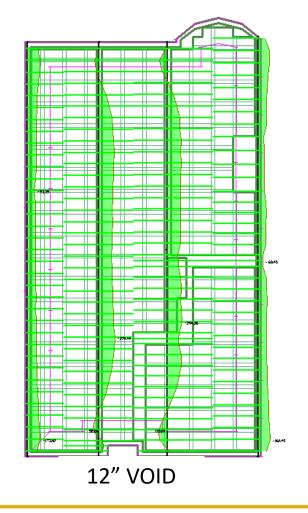




Top Fiber Stresses – Edge Lift – 1 in

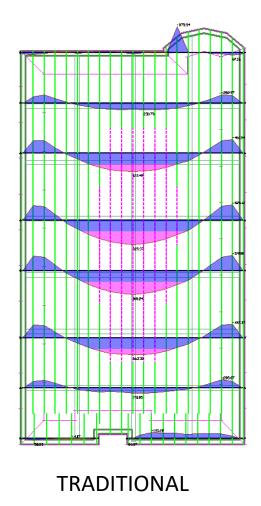


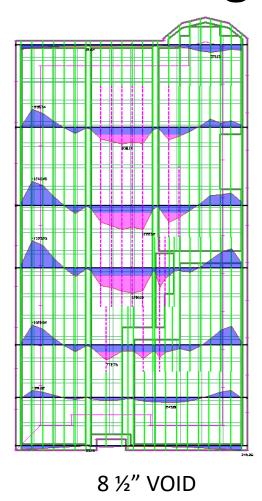


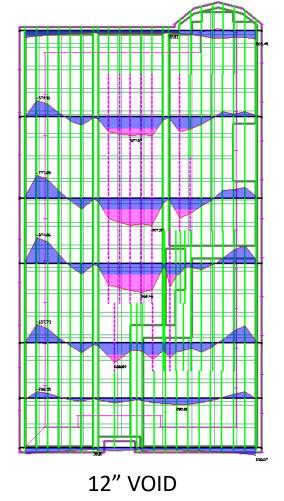




Bottom Fiber Stresses – Edge Lift – 1 in

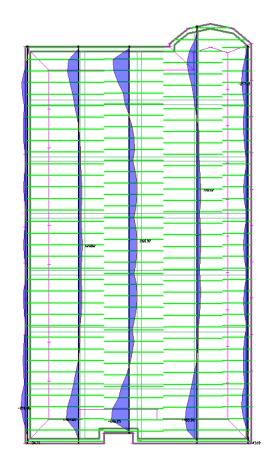


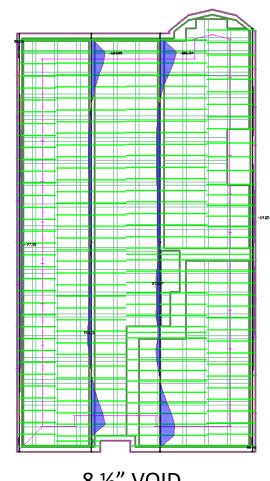


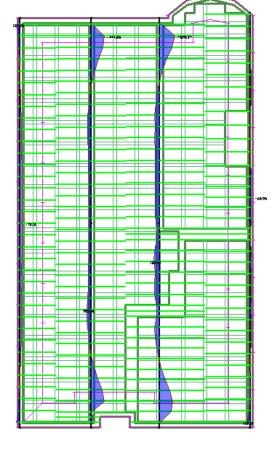




Bottom Fiber Stresses – Edge Lift – 1 in







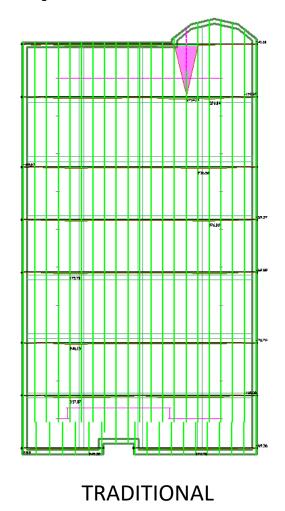
TRADITIONAL

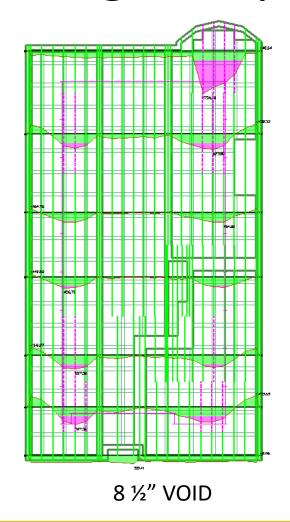
8 ½" VOID

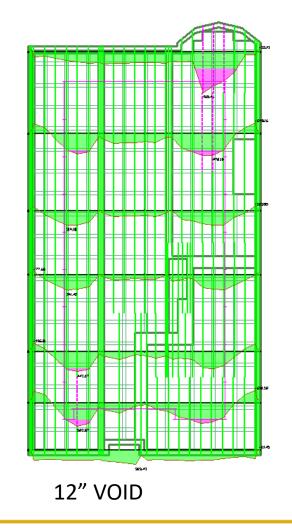
12" VOID



Top Fiber Stresses – Edge Drop – 4 in





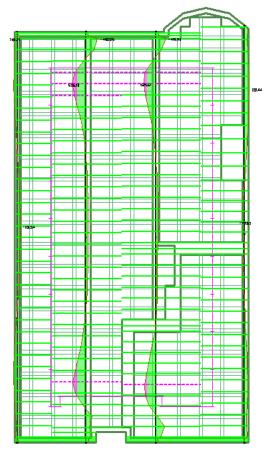




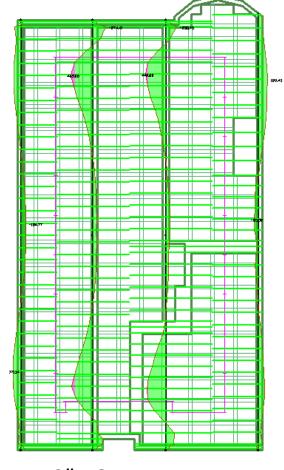
Top Fiber Stresses – Edge Drop – 4 in







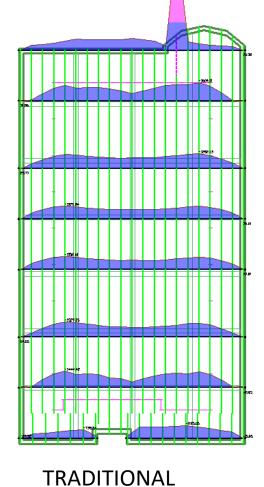
8 ½" VOID

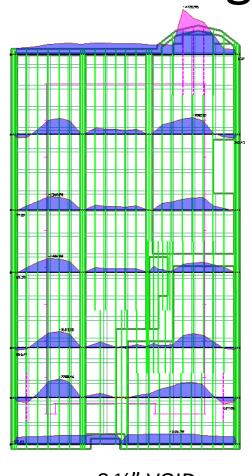


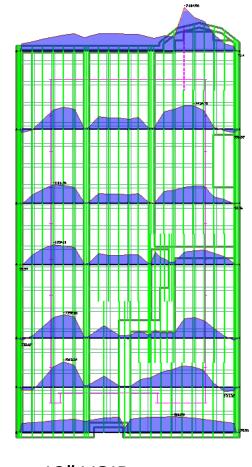
12" VOID



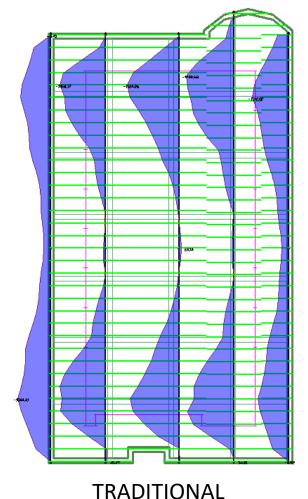
Bottom Fiber Stresses – Edge Drop – 4 in

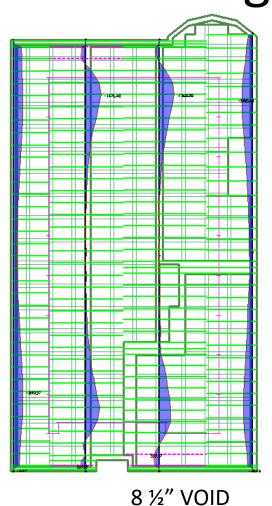


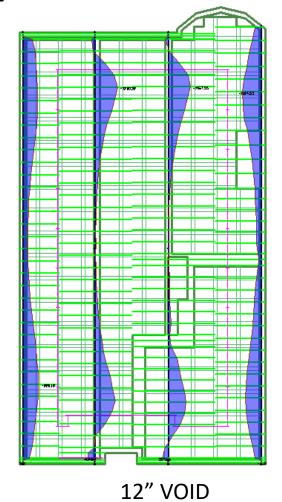




Bottom Fiber Stresses – Edge Drop – 4 in

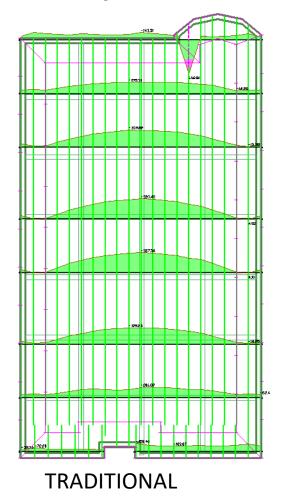


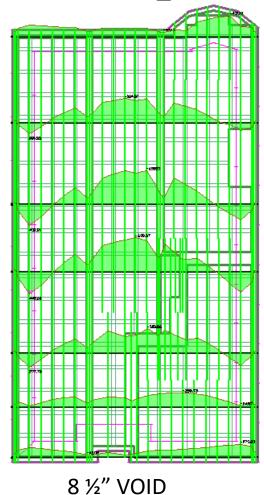


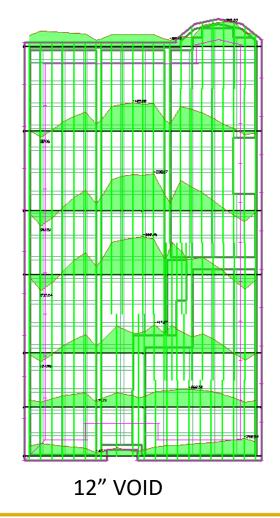




Top Fiber Stresses – Edge Lift – 4 in

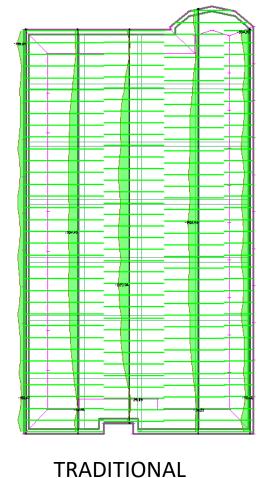


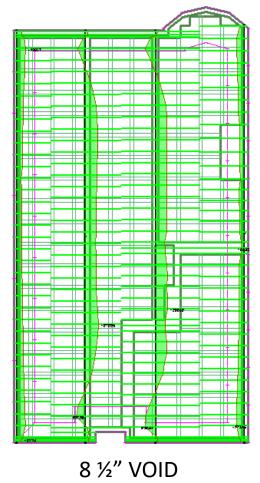


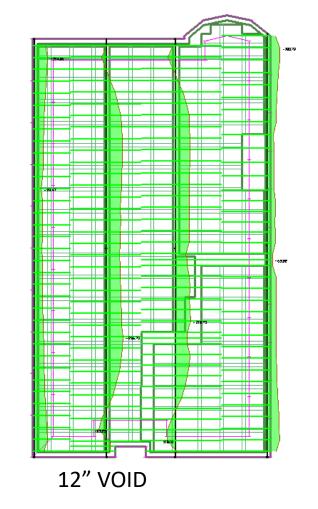




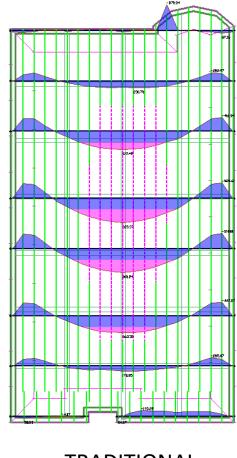
Top Fiber Stresses – Edge Lift – 4 in



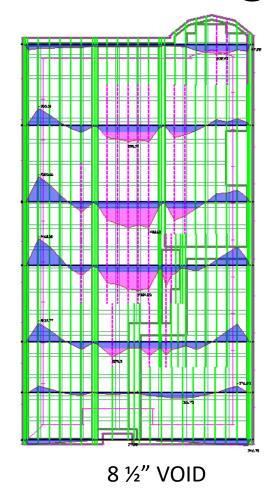


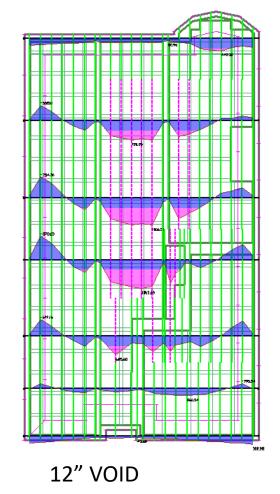


Bottom Fiber Stresses – Edge Lift – 1 in





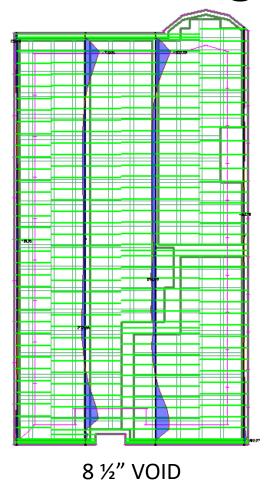


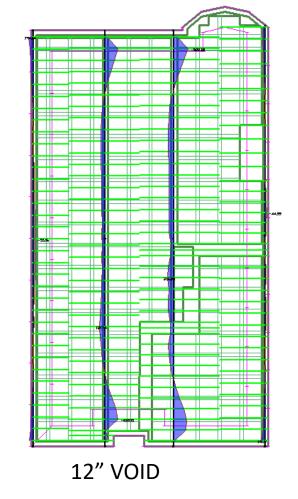




Bottom Fiber Stresses – Edge Lift – 4 in







Holistic Overview of Design Options

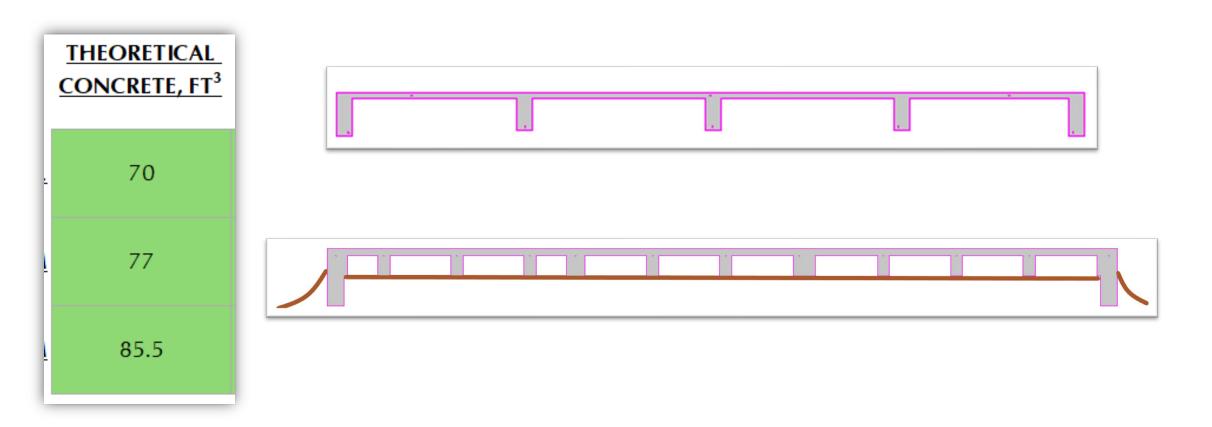
	THEORETICAL CONCRETE, FT ³	<u>PT, LF</u>	PERIMETER BEAM, LF	VOID FORM BOXES, Avg Cost/sq ft*	POTENTIAL CONCRETE WASTE & OVERAGES AT INTERIOR**	INTERIOR BEAMS EXCAVATION & MAINTENANCE	VAPOR BARRIER
TRADITIONAL	70	1,800	225.7	N/A	\$\$\$	\$\$	\$\$
8 1/2"VOID FORM	77	1,800	225.7	\$	N/A	N/A	\$
12"VOID FORM	85.5	1,800	225.7	\$	N/A	N/A	\$

^{*} Based on Market Prices, Prices may vary by region (8.5" boxes is approx \$1.75/ft² & 12" boxes \$2.25/ft²)



^{**}Theoretical volume is increased by waste factors of over excavating all trenched beams (Traditional $\sim 95 \text{ ft}^3$, 8.5" box $\sim 79 \text{ ft}^3$, 12" box $\sim 87.5 \text{ ft}^3$)

Theoretical Concrete





Post Tension Tendons

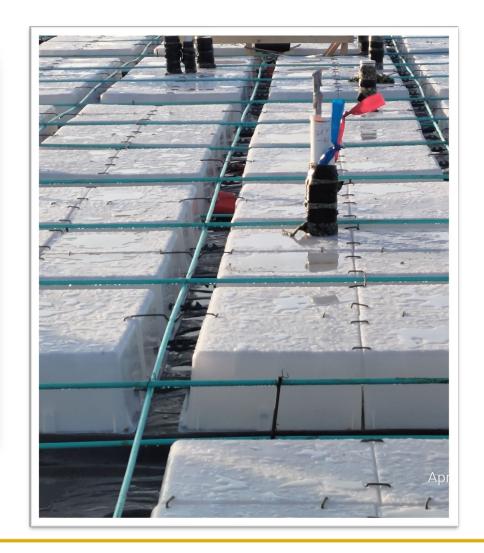


PT, LF

1,800

1,800

1,800





Perimeter Beam

PERIMETER BEAM, LF

225.7

225.7

225.7







Void Form Boxes

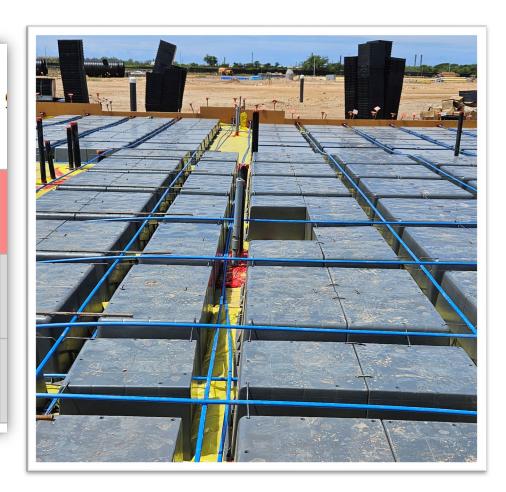


VOID FORM
BOXES, Avg
Cost/sq ft*

N/A

\$

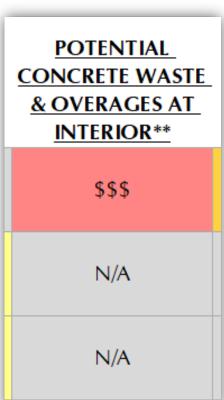
\$





Potential Concrete Waste & Overages at Interior







Interior Beam Excavations & Maintenance



EXCAVATION &
MAINTENANCE

\$\$

N/A

N/A





Vapor Barrier



VAPOR BARRIER

\$\$

\$

\$





Observations

Even though the full cross-sectional moment of inertia was selected to be the same for all foundation models, their ultimate performance is impacted by their asymmetric geometry, precompression, tendon profiling and construction methods.



This concludes the Educational Content of this activity.

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