

Post-Tensioned Transfer Slab in Vertically Irregular Building

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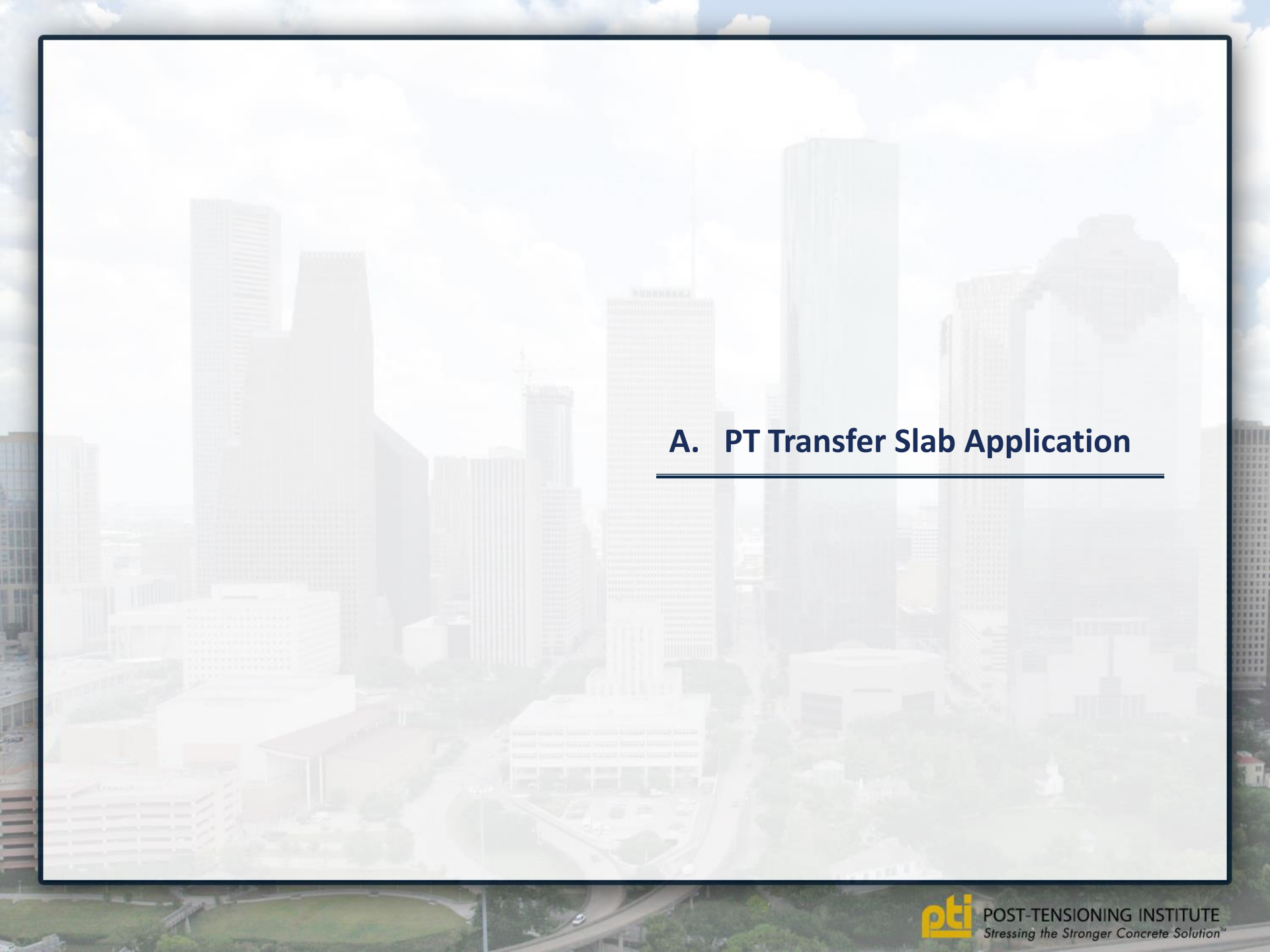
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Technical R&D Institute

DAELIM



POST-TENSIONING INSTITUTE
Stressing the Stronger Concrete Solution™



A. PT Transfer Slab Application



PT Transfer Slab in High-rise Building



The Cullinan, Kowloon Hong Kong (68F)



Belcher Garden, Hong Kong (63F)

Pacific Place Building (VSL Hong Kong, 1988)

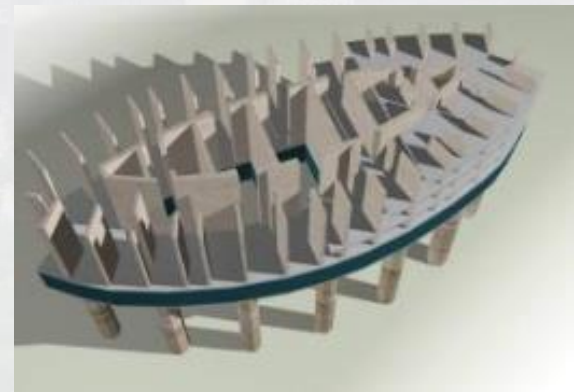


Pacific Place Building (61F)

1st PT Transfer Slab

(Designed by Prof. Peter Marti)

- PT Transfer Slab Thickness 4.5m
- Rebar ($480\text{kg/m}^3 \rightarrow 180\text{kg/m}^3$)
- Better Structural Performance
- Easier Construction
- Economy



Pacific Place Building (VSL Hong Kong, 1988)



Installation of Side Formwork



Installation of Anchorage

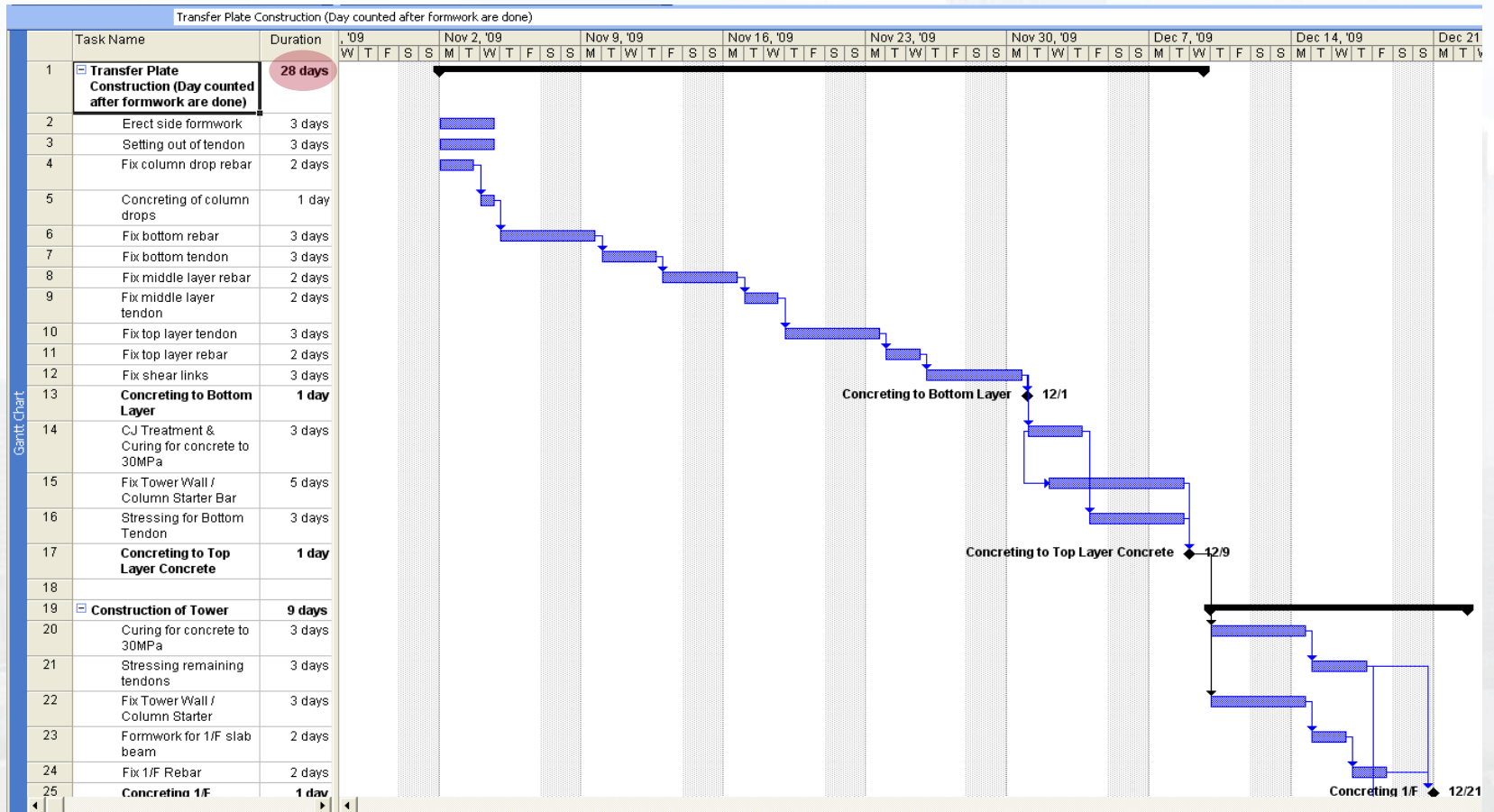


Installation of Reinforcement & Tendon



Installation of Reinforcement & Tendon

Pacific Place Building (VSL Hong Kong, 1988)





B. PT Transfer Slab Design

NEST Hotel (DAELIM, 2014)

Vertically Irregular Structure _ Design Stage



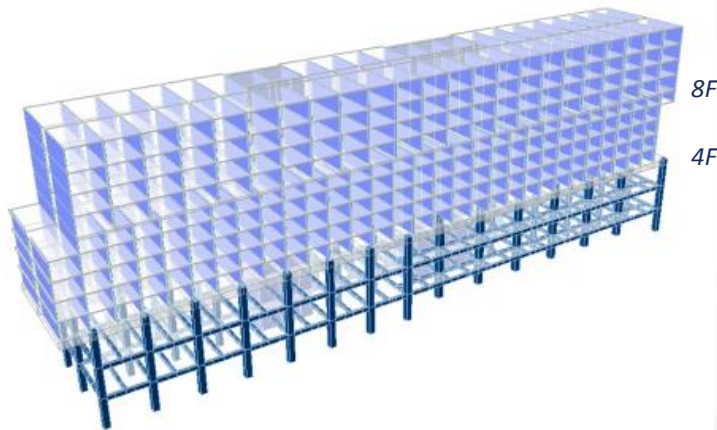
NEST Hotel (DAELIM, 2014)

Vertically Irregular Structure _ Construction Stage



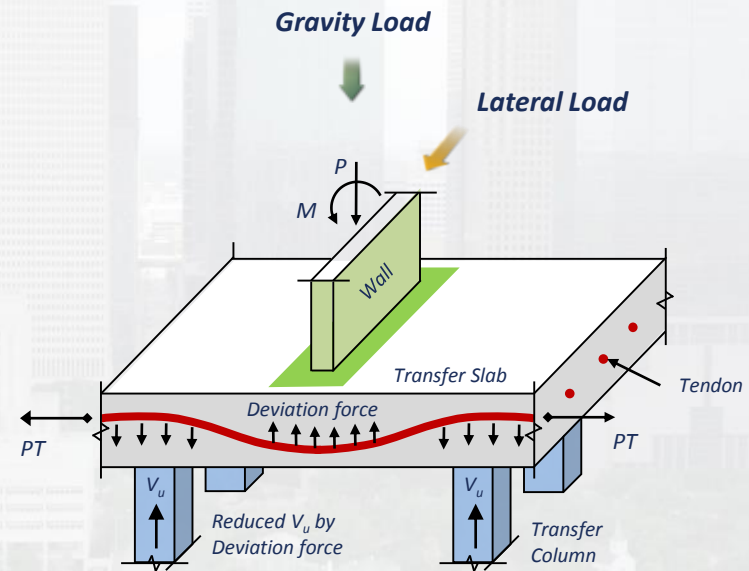
NEST Hotel (DAELIM, 2014)

Post-Tensioned Transfer Slab (Cantilevered)



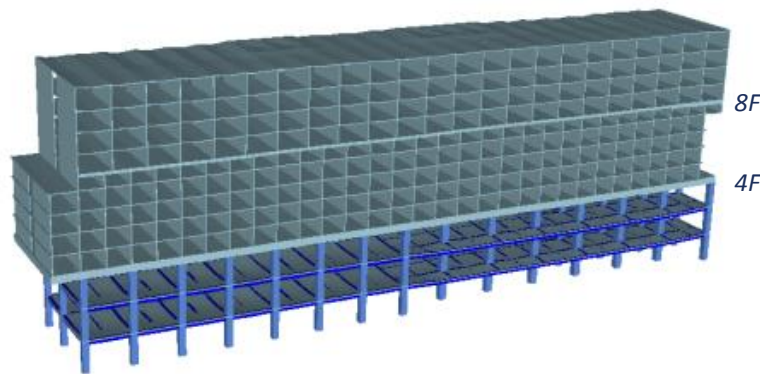
Structural Modeling by MIDAS

Why Post-Tensioning to Transfer Slab ?

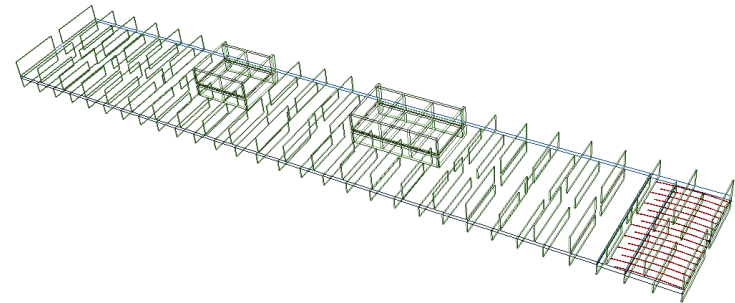


NEST Hotel (DAELIM, 2014)

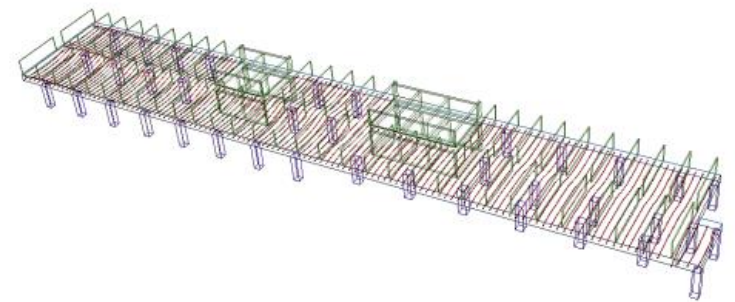
Post-Tensioned Transfer Slab (Cantilevered)



Structural Modeling by ADAPT



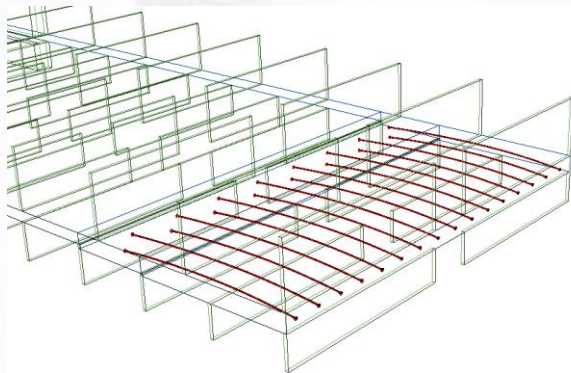
8F_PT Transfer Slab



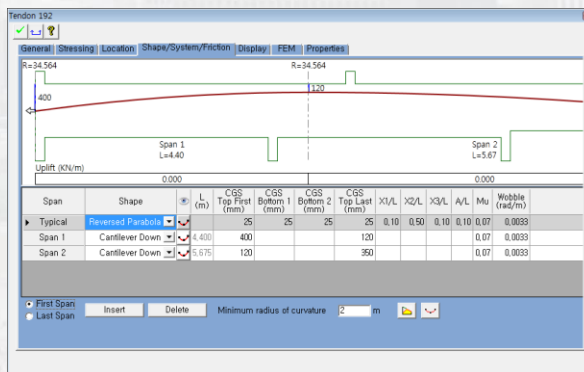
4F_PT Transfer Slab

NEST Hotel (DAELIM, 2014)

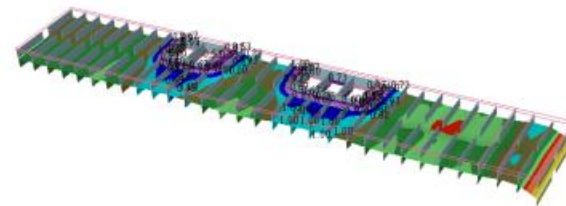
8F Cantilevered Transfer Slab _ Long-term Deflection Check



Distributed Tendon Profile
(10EA_15.2mm@1,200)

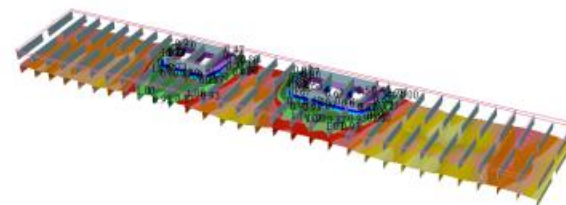


uncracked_Long_Term_RC: Z-Translation[1 Contour = 1.275 mm ; Unit[per contour only]: * 1e+001 mm;
Maximum Value = 2.332e+001 [mm] @ (0.000 6.301 15.300)m;
Minimum Value = 4.190e+000 [mm] @ (73.200 19.450 27.900)m;



RC Slab

uncracked_Long_Term_PT: Z-Translation[1 Contour = 0.693 mm ; Unit[per contour only]: * 1e+001 mm;
Maximum Value = 1.460e+001 [mm] @ (0.000 6.301 15.300)m;
Minimum Value = 4.199e+000 [mm] @ (73.200 19.450 27.900)m;



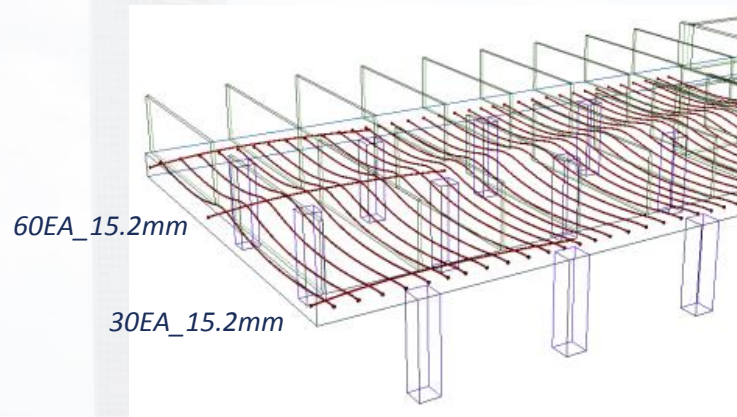
PT Slab

$$\delta = 23.3\text{mm} \rightarrow \delta = 14.6\text{mm} \text{ (-37\%)}$$

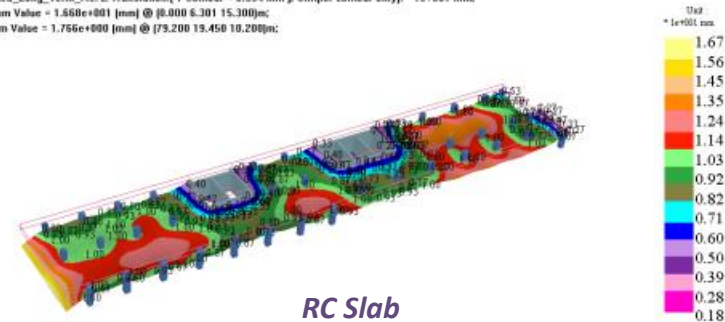
(Limit : $L/240 = 15.6\text{mm}$, $L=3,750\text{mm}$)

NEST Hotel (DAELIM, 2014)

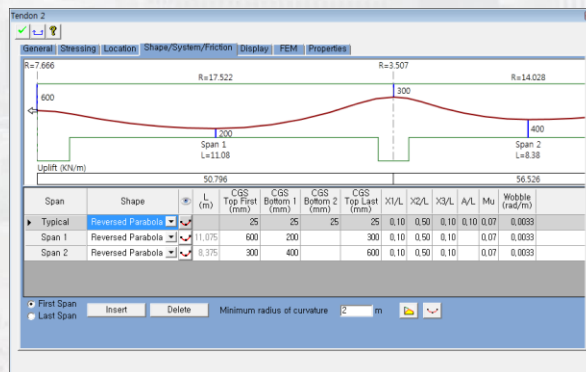
4F Cantilevered Transfer Slab _ Long-term Deflection Check



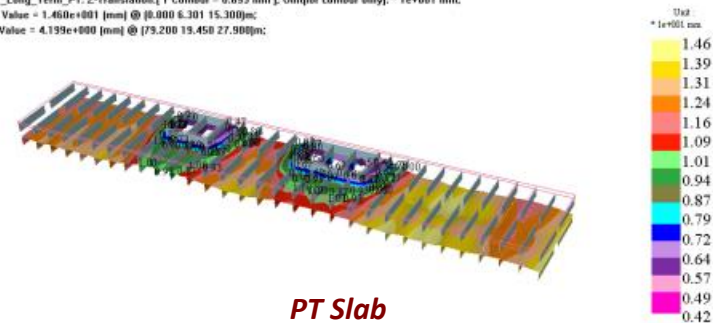
uncracked_Long_Term_RC: Z-Translation: [1 Contour = 0.994 mm ; Unit(For contour only): * 1e+001 mm;
Maximum Value = 1.868e+001 [mm] @ (8.000 6.301 15.300)mm;
Minimum Value = 1.756e+000 [mm] @ (73.200 19.450 10.200)mm;



Distributed Tendon Profile (8EA_15.2mm@1,000)



uncracked_Long_Term_PT: Z-Translation: [1 Contour = 0.693 mm ; Unit(For contour only): * 1e+001 mm;
Maximum Value = 1.460e+001 [mm] @ (8.000 6.301 15.300)mm;
Minimum Value = 4.199e+000 [mm] @ (73.200 19.450 27.900)mm;

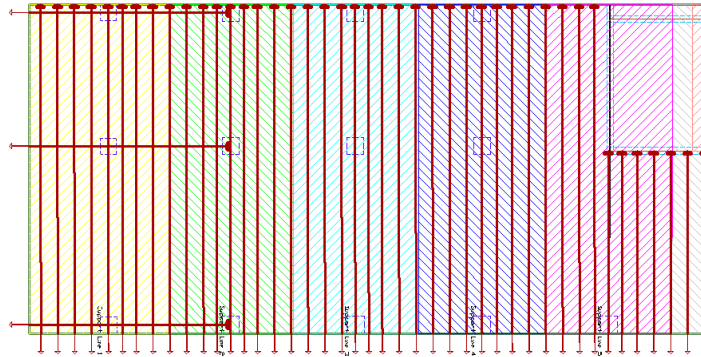


$$\delta = 16.7\text{mm} \rightarrow \delta = 12.6\text{mm} \text{ (-25\%)}$$

(Limit : $L/240 = 17.5\text{mm}$, $L=4,200\text{mm}$)

NEST Hotel (DAELIM, 2014)

4F Cantilevered Transfer Slab _ Initial & SLS Check



< Design Strip _ Y-dir >

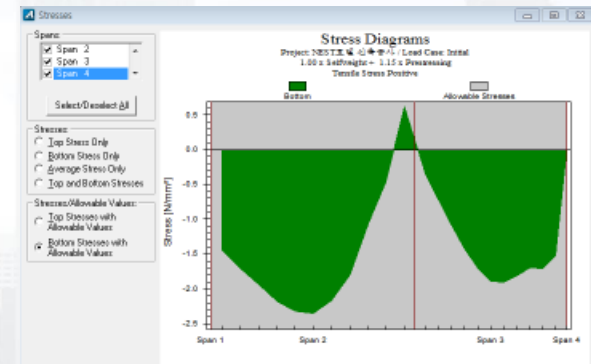
KBC 2012 / ACI Code

1) Initial Stage

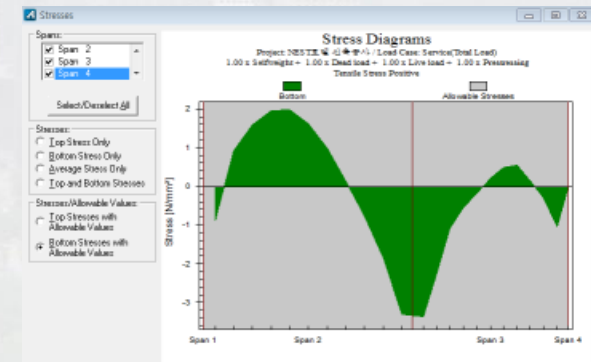
- ① Tensile Stress : $0.25vf_{ci} = 1.17\text{MPa}$ ($f_{ci}=22\text{MPa}$)
- ② Comp. Stress : $0.6f_{ci} = 13.2\text{MPa}$ ($f_{ci}=22\text{MPa}$)

2) Serviceability Stage

- ① Tensile Stress : $0.5vf_{ck} = 2.96\text{MPa}$ ($f_{ck}=35\text{MPa}$)
- ② Comp. Stress : $0.60f_{ck} = 21\text{MPa}$ ($f_{ck}=35\text{MPa}$)



< Bottom Stress _ Initial >



< Bot Stress _ Serviceability >

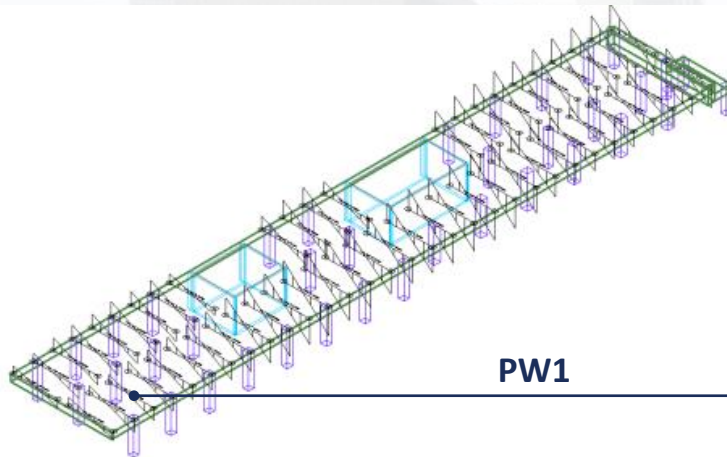
NEST Hotel (DAELIM, 2014)

4F Cantilevered Transfer Slab _ ULS Design

Special Earthquake Load (KBC : Korean Building Code)

$$E_m = \Omega_o \cdot E \pm 0.2 \cdot S_{DS} \cdot D$$

$\Omega_o = 2.5$



ADAPT Wall Load Input

Name	Load	Axial (kN)	Moment (kN·m)	Length (m)	Start (kN/m)	End (kN/m)
PW1	DL	-3,139	-2,751	8.71	-578	-143
PW1	LL	-897	-641	8.71	-154	-52
PW1	RX(ES)	0	-41	8.71	-3	3
PW1	RX(RS)	39	290	8.71	27	-18
PW1	RY(ES)	113	875	8.71	82	-56
PW1	RY(RS)	146	1,348	8.71	123	-90
PW1	WX	-5	-54	8.71	-5	4
PW1	WY	-30	-343	8.71	-31	24

Line Load

✓ ?

General | Loads | Location | Properties |

#:

180

Label:

Line Load 102 RX

Group:

RX RS

Load:

RX_RS

Load

Fz1:

-27,000 kN/m

☒ Variable Magnitude

Fz2:

18,000 kN/m

Mx1:

0,00 kN-m/m

Mx2:

0,00 kN-m/m

My1:

0,00 kN-m/m

My2:

0,00 kN-m/m

Downward load

Moment signs follow global

1 and 2 refer to the start and end of the line

☐ Program generated

NEST Hotel (DAELIM, 2014)

4F Cantilevered Transfer Slab _ ULS Design

Wood Armer Equation (Twisting Moment)

[Ref.] Manual for Design and Detailings of RC to Code of Practice for Structural Use of Concrete 2004 (HK Housing Department)

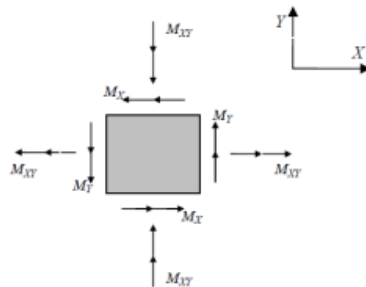


Figure D-3a – General co-existence of bending moments and twisting moment in a plate bending structure

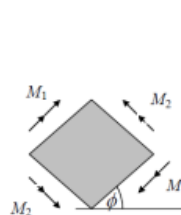


Figure D-3b – Principal moment in a plate bending structure

For bottom steel reinforcement provisions:

$$\text{Generally } M_X^* = M_X + |M_{XY}|; \quad M_Y^* = M_Y + |M_{XY}|;$$

$$\text{If } M_X^* < 0, \text{ then } M_X^* = 0 \quad \text{and} \quad M_Y^* = M_Y + \left| \frac{M_{XY}^2}{M_X} \right|$$

$$\text{If } M_Y^* < 0, \text{ then } M_Y^* = 0 \quad \text{and} \quad M_X^* = M_X + \left| \frac{M_{XY}^2}{M_Y} \right|$$

For top steel reinforcement provisions:

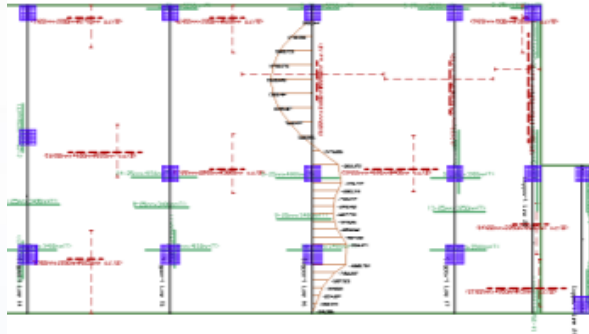
$$\text{Generally } M_X^* = M_X - |M_{XY}|; \quad M_Y^* = M_Y - |M_{XY}|;$$

$$\text{If } M_X^* > 0, \text{ then } M_X^* = 0 \quad \text{and} \quad M_Y^* = M_Y - \left| \frac{M_{XY}^2}{M_X} \right|$$

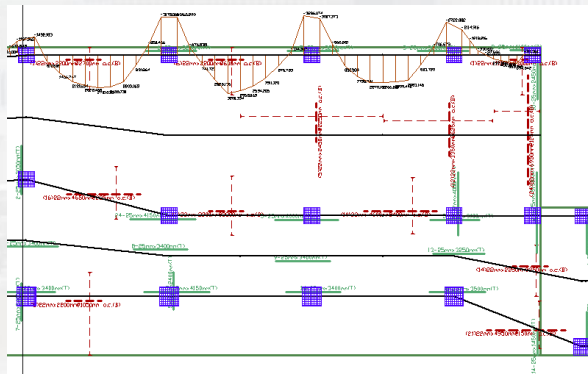
$$\text{If } M_Y^* > 0, \text{ then } M_Y^* = 0 \quad \text{and} \quad M_X^* = M_X - \left| \frac{M_{XY}^2}{M_Y} \right|$$

NEST Hotel (DAELIM, 2014)

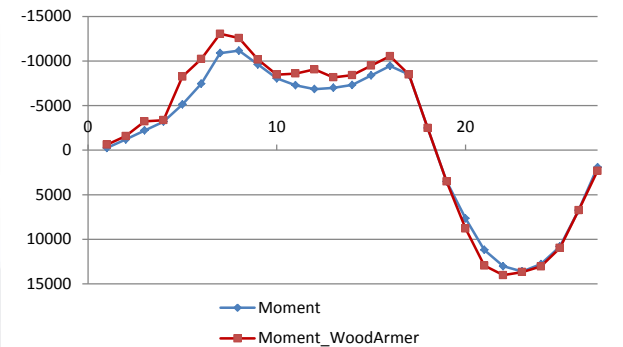
4F Cantilevered Transfer Slab _ ULS Design



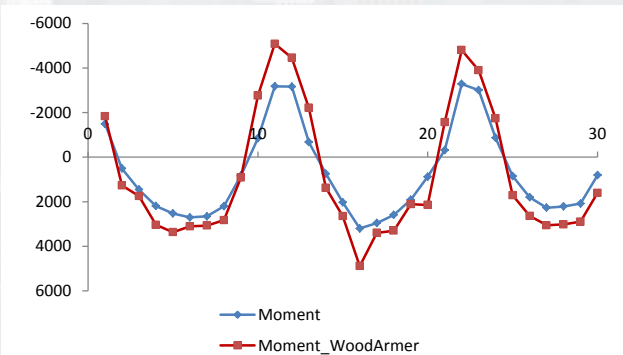
Support Line 16 Moment (Y-Dir)



Support Line 16 Moment (X-Dir)



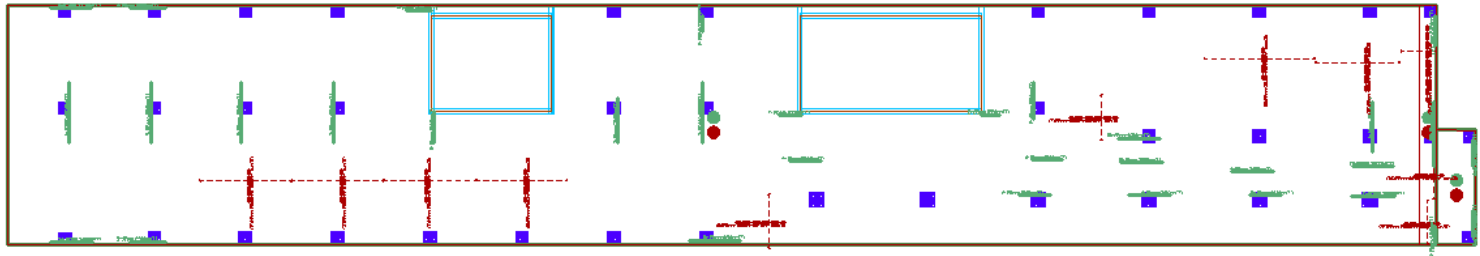
Twisting Moment Increase (Y-Dir)



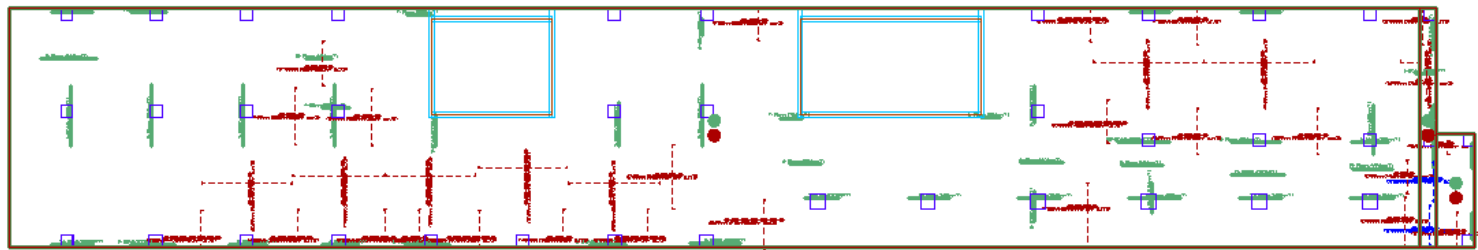
Twisting Moment Increase (X-Dir)

NEST Hotel (DAELIM, 2014)

4F Cantilevered Transfer Slab _ ULS Design



Flexural Reinforcement without Twisting Moment



Flexural Reinforcement with Twisting Moment

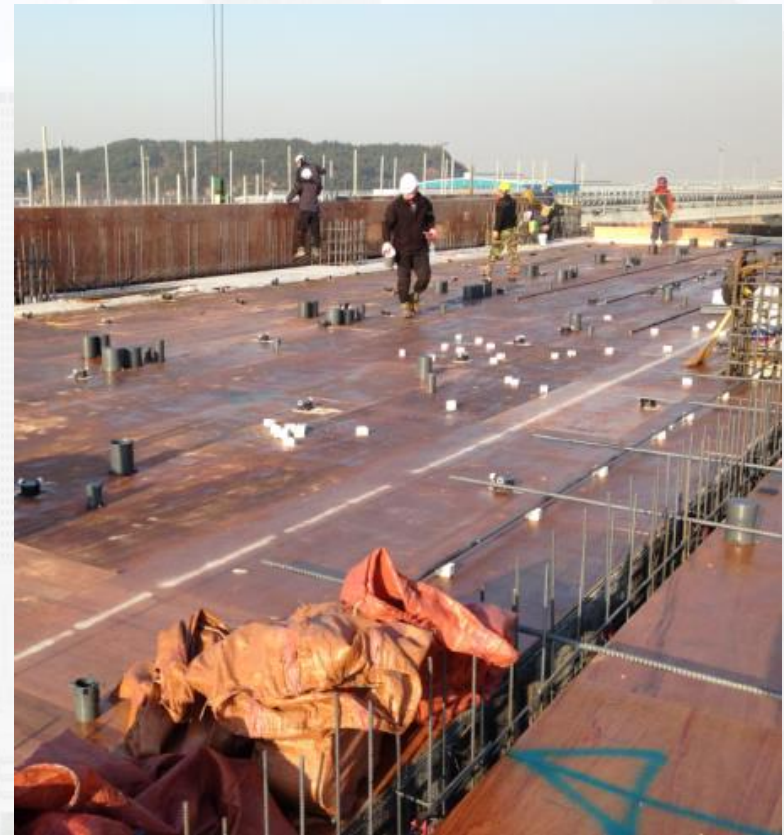
An aerial photograph of a city skyline, likely Los Angeles, featuring prominent skyscrapers like the US Bank Tower. A large, semi-transparent white rectangular box is overlaid on the image, containing the section title. The box has a thin black border.

C. PT Transfer Slab Construction



NEST Hotel (DAELIM, 2014)

Construction _ 01 Formwork



NEST Hotel (DAELIM, 2014)

Construction _ 02 Bottom Rebar



NEST Hotel (DAELIM, 2014)

Construction _ 03 Unbonded Tendon



NEST Hotel (DAELIM, 2014)

Construction _ 04 Distributed Tendon



NEST Hotel (DAELIM, 2014)

Construction _ 05 Concentrated Tendon



NEST Hotel (DAELIM, 2014)

Construction _ 06 Anchorage Reinforcement



NEST Hotel (DAELIM, 2014)

Construction _ 07 Top Rebar



NEST Hotel (DAELIM, 2014)

Construction _ 08 Punching Shear Reinforcement



NEST Hotel (DAELIM, 2014)

Construction _ 09 Tendon Stressing



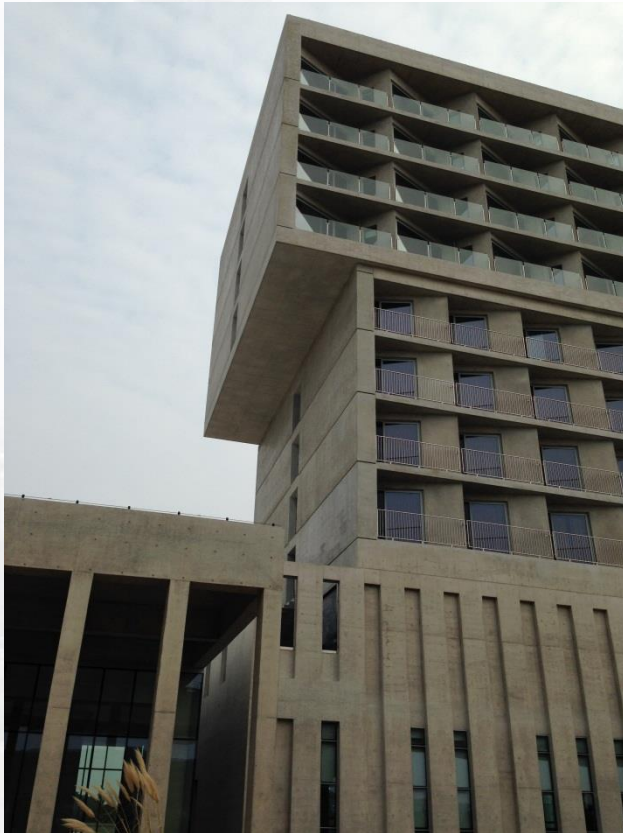
NEST Hotel (DAELIM, 2014)

Precast Wall above PT Transfer Slab



NEST Hotel (DAELIM, 2014)

Vertically Irregular Building





THANK YOU