2016 PTI Convention Long Beach, California



Technical Session 1 Bridge Design and Construction



New Gerald Desmond Cable Stay Bridge Replacement



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OST-TENSIONING √STITUTE ™

Presentation Goals:

- History of Existing Bridge
- Replacement Bridge
 - New vs. Old
 - Award, Schedule, Cost
 - Design Build
 - New Bridge Details
 - Various Construction Methods
 - Heavy Lifting Pier Tables
 - Access Deck & Pylon
 - Foundation Method Tip Grouting
 - Balanced Cantilever Construction Main Span
 - MSS (Mobile Scaffolding System) Approaches





History of Existing Bridge

- Steel Arch Bridge
 - Ground breaking in 1965
 & completed in 1968
 - Clearance is 155 Ft
 - Main Span is 410 Ft



- Key link connecting Port of Long Beach to surrounding area and 710 Freeway
- Approximately 15% of nations containerized water cargo crosses the bridge
- Has become unable to accommodate larger container ships underneath its span & the greater traffic across its deck

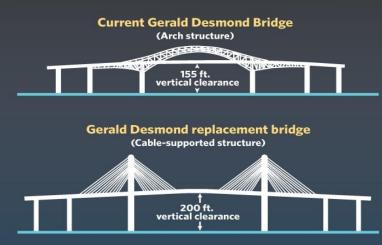


Building a New Bridge

- New vs. Old
- Ships are now 6 times larger than in 1968
- Affecting other bridges
 - Bayonne Bridge in NY
 - Panama Canal-
 - large containers

Building a new bridge

The Gerald Desmond Bridge is an arch bridge originally built in 1968 that carries four lanes of traffic across the Cerritos Channel between Terminal Island and Long Beach. Over the years the bridge has developed maintenance issues and the Port of Long Beach plans to replace it.



The new Gerald Desmond Bridge will feature six traffic lanes and higher clearance for larger ships. There are also certain advantages to the replacement. The structure is economical, strong yet flexible, slender and lighter, which is an advantage in earthquake-prone areas such as California.

Sources: OPAC Consulting Engineers Inc.; Port of Long Beach

Paul Penzella Staff Artist



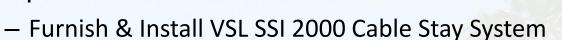
New Bridge Award, Schedule, Cost

- Owner Port of Long Beach
- Design Build awarded to SFI in 2012
 - Shimmick Construction Co, Inc; FCC Construction;
 S.A. Impregilo S.P.A.
- Original Completion was 2016
 - Towers were redesigned
- New Completion is now 2018
- Final Cost is anticipated to be around 1.5 billion



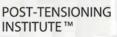
New Replacement Bridge Design

- Design Build
- Cable Stays/Armor/Fire Protection
 - Awarded to VSL
 - Scope-



- Furnish & Install VSL Friction Dampers
- Furnish & Install VSL/Hardwire Armor & Fire Protection
- Stay Installation is schedule to begin in November 2016 with completion in June 2017
- Armoring will begin in June 2017 with completion by Sept







Integrated Stay Cable Solution

Stays | Armor | Fire







Armor/Fire Protection

- VSL will design, fabricate and install armor and fire protection on GDB
- This is becoming more of standard for cable supported bridges - 3 under contract presently
- PTI now requires fire protection on all cable stay bridges
- Protection starts at the roadway and terminates at a certain height above the deck





New Replacement Bridge Design

- Multi-strand Post-Tensioning
 - Awarded to DSI
 - Approaches



- Longitudinal & Bent Cap Tendons- 27 x 0.6" strands
- Transverse tendons in Deck 4 x 0.6" strands
- Main Span Precast Tendons 12 x 0.6" strands





New Replacement Bridge Details

- Cable Stay Bridge Details
 - 205 Ft. of clearance off water
 - 40 Ft. higher than existing bridge
 - Main Span is 1000 Ft. long (two towers)
 - Stay support length is 2000 Ft
 - Longer approaches to accommodate increased height
 - Towers are 310 Ft high from deck





New Replacement Bridge Details

- 80 Stays Total
- Longest stay- 572 ft
- Shortest stay- 236 ft
- Sizes range from 6-43 to 6-85



- Bike & pedestrian path on one side of bridge
- Scenic overlook
- Six lanes vs. four lanes on existing bridge
- Main Span is steel box girder with steel floor beams with precast panels.
- Steel field sections to be fully assembled and brought in by barge and erected with lifters supplied by Deal



New Replacement Bridge Details





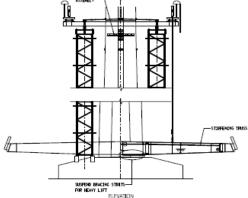
- Balanced cantilevered construction- Main Span – (one segment out of balance)
- Strand by Strand Installation
- AMS (Automatic Stressing)

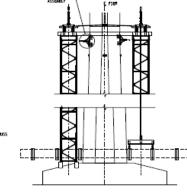






• Heavy Lifting of Pier Tables

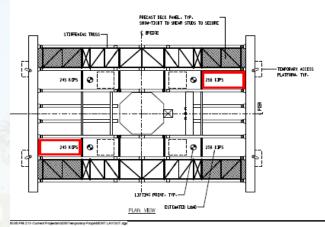


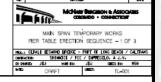


SIDE VEW

POER TABLE EFFECTION SEGMENCE (1 OF 3)

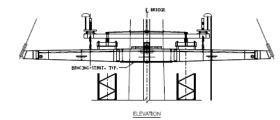
- 1-1- PRE-ERECT SHEAR REY IN TOMER FACES
- 1.2. EPECT HEAVY-LIFT TOWERS (FREE-STANDING)
- 1.3. ERECT HEADER BEAKS AND BRACE TO TOWER.
- 1.4. ERECT FIELD SECTION "T" ON GROUND (INCLUDES STIFFENDIG FRAME AND BRACING).
- SUSPEND BRACING STRATS TO FACILITATE INSTALLATION AT HEIGHT-
- NOTE: STEPS 1-5 MAY BE SDAULTANEOUS OR ANY SECIENCE.
- 1.6. DISTALL PRECAST PANELS AT CORNERS OF FUELD SECTION "T" FOR STAY ACCESS. SIDA PANELS TIGHT AGAINST SIEAR STUDS TO SECURE.
- 1.7. DISTALL TEMPORARY ADJESS PLATFORMS AT STAY ANCHORS.

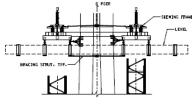






Heavy Lifting of Pier Tables

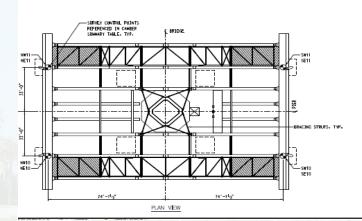




SIDE VIEW

POER TABLE ERECTION SEQUENCE (2 OF 3)

- 2.1. USE STRAND JACKS TO LEFT FEELD SECTION "T" (LEVEL).
 - NOTE: REFER TO SHEET TW-003 FOR CAMBER / ELEVATION ADJUSTMENTS-
- 2.2. INSTALL SAFETY BARS.
- 2.3. INSTALL BRACENG STRUTS.
- 2.4. PERFORM SURVEY. LOOSEN BRACING STRUTS AND USE SKEWING FRAME TO ADJUST FOR LINE AND LONGETUDENAL OFFSET.
- 2.5. TICHTEN BRACING STRUTS AFTER ANY ADJUSTMENT.
- 2.6. PERFORM CONFERNATION SURVEY.
- 2.7. INSTALL AND STRESS CABLE PAIRS 10 AND 11 PER THE STAY STRESSING SCHEDULE (THIS SHEET).
 - NOTE: RE-CHECK AND TIGHTEN BRACING STRUTS DURING STAY STRESSING.
- 2.8. RELEASE ANY RENAINING LOAD IN THE LIFTING JACKS.
- 2.9. REMOVE SAFETY BARS.
- 2.10. CUT OFF PADEYES.



STAY ST	TRESSING SCHEDULE				
PH4SE	PLER	6 N#10	S#10	N#11	S#11
	PEER	7 NE10	SELO	NEI 1	SEI 1
2.7. 5	STRESS STAYS (INITIAL)				
(STRESS TO FORCE - GRADUATED INCREMENTS?				
1	TOTAL FORCE (KJPS)	265	271	265	271
,	NO. STRANDS	42	44	42	- 44
1	1ST STRAND (KIPS)	18.60	19.05	18.60	19.05
1	INCREMENT (REPS)	-0-30	-0-30	-0-30	-0.30
L	LAST STRAND (KIPS)	6.31	6.16	6.31	6.16
,	AVG. / STRAND (K]PS)	6.31	6.16	6.31	6.16
3.4. 8	PLACE PRECAST DECK PANELS				
((PASSIVE LOADING)				
۱	TOTAL FORCE (KIPS)	607	614	607	614
	AVG. / STRAND (KIPS)	14.45	13.95	14.45	13.95
3.5.	RE-STRESS STAYS 11 FOR GRADE +				
(STRESS TO ELONGATION: 8 PASSES e 2.78" - 22.	20")			
1	TOTAL FORCE (KJPS)	607	614	607	614
,	AVG. / STRAND (KIPS)	14.45	13.95	14.45	13.95
3.7. 1	RE-STRESS STAYS 10 & 11 +				
(STRESS FOR GEOMETRY: NOWINAL STROKE = 1.50"				
1	TOTAL FORCE (KIPS)	607	614	607	614
,	AVG. / STRAND (KIPS)	14.45	13.95	14.45	13.95
+ FUR N	NOTED PHASES+ STRESS ALL STRANDS TO THE SAME	ELONGATION.			-
MAXEN	MUN UNBALANCE FOR NORTH AND SOUTH STAYS = 2 S	TRANDS.			

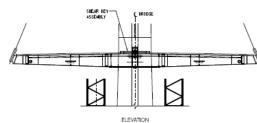


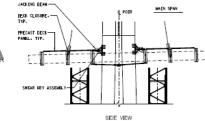
MAIN SPAN TEMPORARY WORKS PIER TABLE ERECTION SEQUENCE - 2 OF 3

PRNUL GERALD DESMOND BREDGE - PORT OF LONG BEACH / CALTRANS CONTRACTOR: SHJIMAJOK / FCC / ENPRECIELO, A J.V. IN CHARGE JBJ MADE EN JDJ CHECK EN WSM



• Heavy Lifting of Pier Tables





POER TABLE ERECTION SEQUENCE (3 OF 3)

- 3.1. REMOVE HEADER BEAMS AND TOWER SECTIONS THAT EXTEND ABOVE DECK LEVEL.
- 2. PEPFORM SURVEY. MAKE FENAL ADJUSTMENTS FOR LINE AND LONGETUDENAL OFFSET USING BRACENG STRUTS.

3.3. INSTALL PRECAST PANELS PER THE PANEL PLACENG SEQUENCE (THIS SHEET).

NOTE: AFTER PLACING FIRST PANEL, MADE "1" GAP AT BACK-SPAN BRACING SRUTS. VERIFY GAP AFTER EVERY PANEL IS PLACED.

3.4. REINFORCE AND CAST DECK CLOSURES.

3.5. RE-STRESS STAY PACE 11 PER THE STAF STRESSING SCHEDULE (TW-002).

NOTE: RE-CHECK AND TIGHTEN BRACING STRUTS DURING STAY STRESSING.

3.6. PERFORM SURVEY-

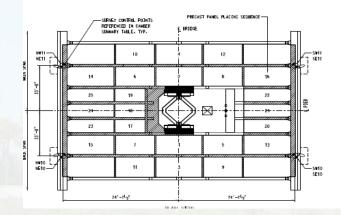
3.7. RESTRESS ALL STAYS FOR THE STAY STRESSING SCHEDULE FOR REDUCTED DEECTION ELEVATIONS (NONIONAL STROKE = 1.5").

3.8. FORM AND REINFORCE JACKENG BEAM / SHEAR REY.

3.9. AFTER JACKING BEAM / SHEAR KEY CONCRETE REACHES 4000 PS1. RELEASE AND REMOVE BRACING STRUTS.

3.10. PROCEED WITH DECK ERECTION.

3-11. AFTER EXECTING ADJACENT FIELD SECTIONS AND CASTING DECK CLOSURES. THE STIFFENENG TRUSSES MAY BE REMOVED.



CANBER SUMMARY	INCREMENTAL DEFLECTIONS (FT)			ELEVATION ADJUSTMENTS (FT)					
PH4SE	PEER 16	N#10	S#10	NICE 1	SWI 1	N#10	S#10	N#11	S#11
	PEER 17	NE10	SE10	NE11	SEI 1	NEIO	SELO	NE11	SE11
2.1. LIFT STEEL		-0.400	-0.417	-0.400	-0.417	-0.025	-0.053	-1.792	-1.793
2.7. STRESS STAYS (JNJ)	(JAL)	+0.400	+0.417	+0.400	+0.417	+0.375	+0.364	-1.392	-1.377
3.4. PLACE PRECAST DECI	PANELS	-0.270	-0.259	-0.270	-0.259	+0.104	+0.105	-1.662	-1.635
3.5. RE-STRESS STAYS 1	FOR GRADE	0.000	0.000	+1.740	+1.740	+0.104	+0.105	+0.078	+0.104
3.7. RE-STRESS STAYS 1	0 & 11	+0.118	+0.118	+0.118	+0.118	+0.222	+0.222	+0.195	+0.222
SUPERIMPOSED DEFLI	ECTIONS	-0.222	-0.222	-0.195	-0.222	0.000	0.000	0.000	0.000
TUTAL		-0.375	-0.364	+1.392	+1.377				
CANEER		+0.375	+0.364	-1.392	-1.377				



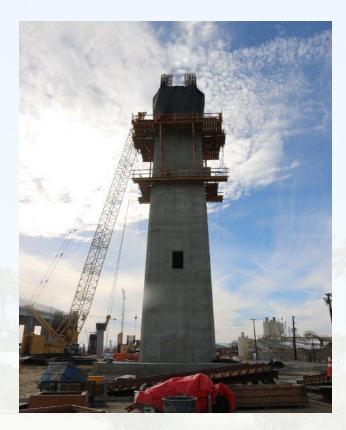
• Tower Crane at Each Pier - Access

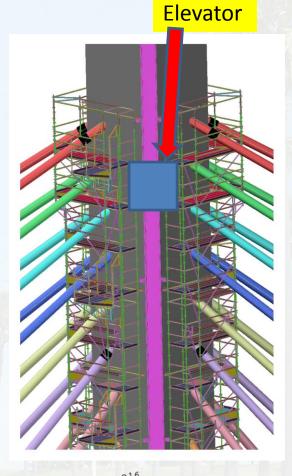




Fully Scaffold Towers - Access

 with elevator for stay install

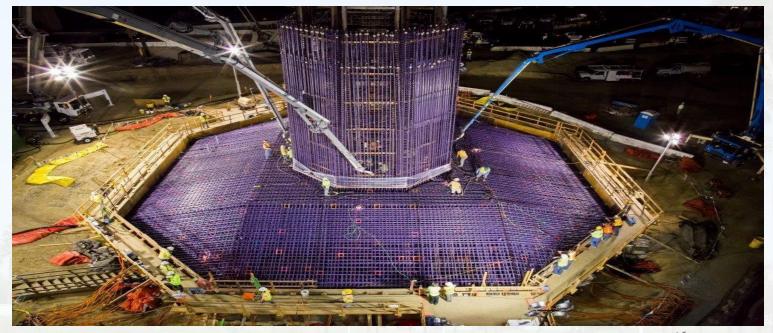






Foundations

- 3200 yards of concrete in pier foundations
- Continuous pour for 24 hours





- Tip Grouting on Foundations
 - Not common on Caltrans Projects
 - Used on approximate 6 projects in US
 - John James Audubon Bridge
 - » Article by Steven Dapp, Ph.D., P.E.
 - » Dan Brown, Ph.D., P.E.
 - Benefits of Tip Grouting
 - Increases the ultimate tip capacity
 - Tip is able to contribute to useful capacity
 - Provides proof load of capacity for all shafts on site



- When to Consider Tip Grouting
 - Sandy bearing stratum
 - Shaft tip is difficult to clean or keep clean
 - Increase reliability- proof load every shaft
 - Gaining acceptance in US
- Mass Concrete Pours
 - Majority of foundation and bent cap pours utilized a cooling system

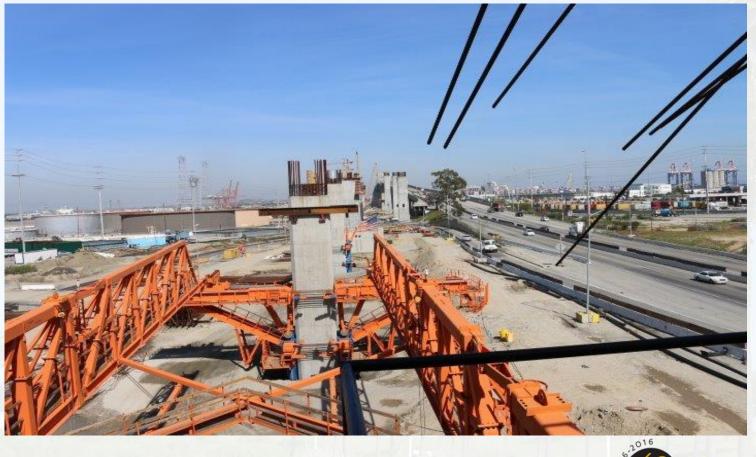


• MSS (Mobile Scaffolding System)



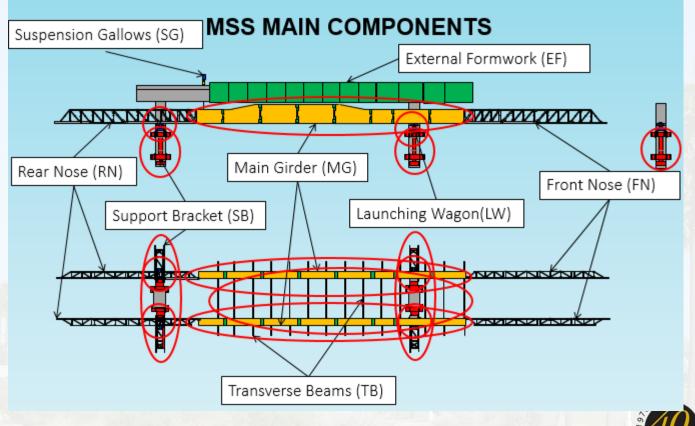


• MSS – Used on E&W Approaches





• MSS







• MSS

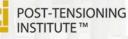


Before August, 2015



<u>After</u> December, 2015





- Movable Scaffolding System MSS
- MSS replaces conventional falsework means and methods when constructing in unsuitable ground conditions and heights greater than 150 feet
- Approximately 50,000 bolts per MSS, 22,000 of the total are torqued
- Approximately 14,200 pieces, each marked with their own unique I.D.



- MSS first time used in North America
- MSS Video for First Launch



Project Team

- Project Team:
 - Owner
 - General Contractor
 - Design Engineer
 - Construction Engineer
 - Stays
 - PT

Port of Long Beach
SFI JV
Arup
Arup/McNary Bergeron
VSL
DSI



Thank You!

Questions?

